## DA117/2017 Proposed Residential Aged Care Facility

## 266 Longueville Road, Lane Cove

#### REVISED TRAFFIC AND PARKING ASSESSMENT REPORT

28 March 2019

Ref 17311



## **Executive Summary**

This revised report has been prepared to accompany an amended Development Application to Council (DA117/2017) for a new seniors living development to be located at 266 Longueville Road, Lane Cove.

On Wednesday 11<sup>th</sup> July 2018, the *Sydney North Planning Panel* meeting was held at Lane Cove Council to discuss a new seniors living development proposal on the subject site, comprising 82 independent living units (ILUs), a 70-bed residential aged care facility (RACF), a two-level basement car parking area for 122 cars, plus a loading bay and ambulance bay.

Due to a number of concerns raised by local residents the Panel chose to defer the matter, subject to additional information being provided by the Applicant, including obtaining an independent peer review of the traffic and parking assessment report prepared by *Varga Traffic Planning* (*VTP*).

Council therefore engaged *Bitzios Consulting (Bitzios)* to undertake the peer review of the traffic study who recommended that a number of modifications were required of the SIDRA traffic model, including taking into account the RMS's concept signal plan for realignment works at the River Road West & Longueville Road intersection, allowing right turn movements from Longueville Road onto River Road West for *all* traffic.

It is pertinent to note that VTP only received a copy of the RMS's concept signal plan the afternoon of the Panel meeting, such that the VTP traffic model was based on the abovementioned right turn movements from Longueville Road onto River Road West restricted to buses only.

Accordingly, the VTP SIDRA traffic model has been revised to take into account the Bitzios peer review comments, including allowing right turn movements from Longueville Road onto River Road West for all traffic. It is also worth noting that whilst the Bitzios peer review traffic study provided a number of recommendations, no SIDRA files or results were provided to VTP for cross-scrutiny.

In essence, the revised capacity analysis reconfirms that the traffic generation potential of the development proposal on the subject site will not result in the reduction in the *Level of Service* of the nearby intersections.

Other modifications to the design, from a traffic and parking perspective, include a new 2m wide landscaping strip along the southern boundary of the site to improve privacy to the adjacent "Timber Tops" residential development, as recommended by the Panel. In doing so, the site access driveway has shifted 2m to the north.

In addition to the *Bitzios* peer review which was commissioned by Council, "Timbertops" residents engaged *ML Traffic (ML)* to also undertake a peer review of the *VTP* report. Notwithstanding, the *ML* peer review was based on an earlier version of the *VTP* report and therefore outdated information, as well as questionable and incorrect assumptions on certain matters.

In summary, the amended Development Application will not result in any unacceptable traffic implications and complies with the applicable numerical off-street parking requirements.

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### 1. INTRODUCTION

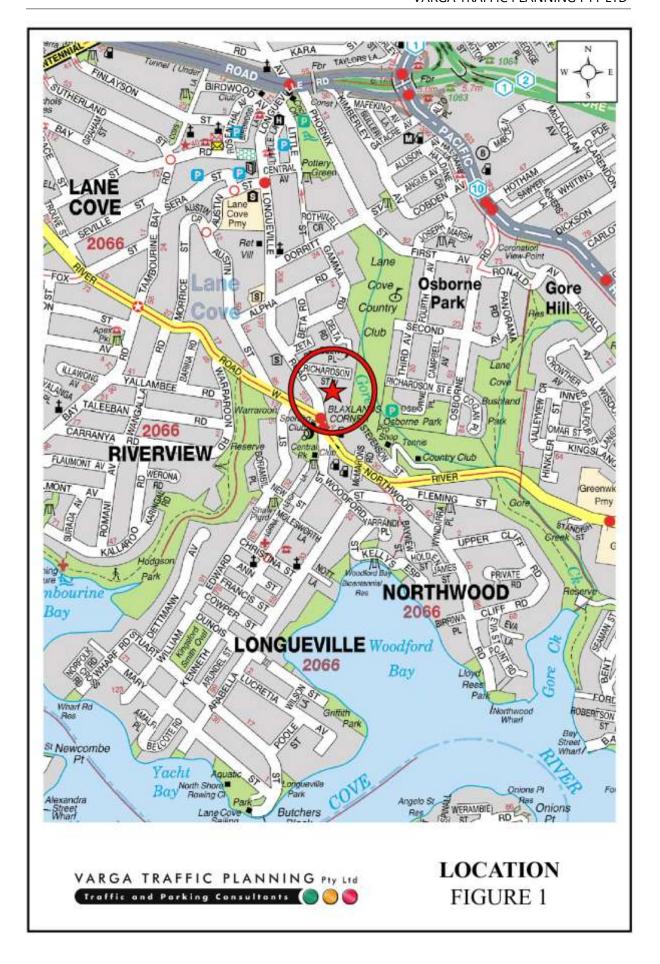
This revised report has been prepared on behalf of *Australian Unity* to accompany an amended Development Application to Council for a seniors living development to be located at 266 Longueville Road, Lane Cove (Figures 1 and 2).

The proposed development involves the demolition of the former lawn bowls facilities and associated car parking area on the site to facilitate the construction of a new seniors living development, comprising a 70-bed residential aged care facility as well as 82 independent living units.

Off-street car parking is to be provided for a total of 122 spaces, plus an ambulance bay, in a new two-level basement parking area in accordance with *State Environmental Planning Policy (Housing for Seniors or People with a Disability) 2004* and Council's requirements.

The purpose of this revised report is to assess the traffic and parking implications of the amended development proposal and to that end this report:

- describes the site and provides details of the development proposal
- reviews the road network in the vicinity of the site
- reviews the public transport services available in the vicinity of the site
- estimates the traffic generation potential of the development proposal
- assesses the traffic implications of the development proposal in terms of road network capacity
- reviews the geometric design features of the proposed car parking and loading facilities
   for compliance with the relevant codes and standards
- assesses the adequacy and suitability of the quantum of off-street car parking and loading provided on the site.





### 2. PROPOSED DEVELOPMENT

### Site

The subject site is located on the eastern side of Longueville Road, approximately 100m south of Richardson St West. The site has a street frontage approximately 101m in length to Longueville Road and occupies an area of approximately 9,200m<sup>2</sup>.

The subject site was previously occupied by two lawn bowls greens, a club house building and an associated car parking area. The bowling greens are no longer used, and the existing building on the site is currently used to accommodate the Lane Cove Music and Cultural Centre. A recent aerial image of the site and its surroundings is reproduced below.



Car parking is currently provided in two separate areas on the site, comprising a small informal car parking area located to the rear of the existing building on the site, and a larger car parking area at the north-western corner of the site which is used primarily for boat trailer storage.

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Vehicular access to the site is provided via two separate driveways which are located adjacent

to the southern boundary of the site and also midway along the site frontage, further to the

north in Longueville Road.

The southern vehicular access driveway in Longueville Road is shared with the adjacent

residential apartment building known as "Timber Tops" which is located immediately to the

south of the subject site.

**Proposed Development** 

The proposed development involves the demolition of the former lawn bowls facility and

associated car parking area on the site to facilitate the construction of a new seniors living

development.

A total of 82 independent living units (ILU's) are proposed as follows:

1 bedroom apartments:

11

2 bedroom apartments:

71

TOTAL APARTMENTS:

82

A total of 70 residential aged care (RACF) beds are also proposed within the new building.

There will be a maximum of 25 staff during the main day shift, with 34 staff on-site during

the afternoon shift change. In this regard, the afternoon shift change occurs between 2:45pm-

3:00pm however staff often arrive up to 30 minutes earlier whilst staff often leave up to 30

minutes *later* (typical of many workplaces) – i.e. not all staff will arrive and depart the site

during the abovementioned 15 minute "window", in reality those movements would likely be

distributed across a full hour period.

A number of ancillary facilities are proposed for the use of residents, including several

common areas such as dining rooms, TV lounges and a courtyard. A number of back-of-

house areas are also proposed, such as an administration/community facilities office, laundry,

kitchen, staff lockers and a maintenance room.

5

Off-street parking is proposed for a total of 122 cars, plus an ambulance bay, in a new two-level basement car parking area in accordance with *SEPP 2004* and Council's requirements. The amended scheme now also makes provision for ILU visitor parking within the basement.

A porte-cochere is also proposed to facilitate the drop-off and pick-up of (mostly elderly) passengers, with entry via a one-way driveway located midway along the site frontage and exit via the main site access driveway.

Vehicular access to the site is to be provided a new entry/exit driveway which is to be located towards the southern end of the Longueville Road site frontage, near the *existing* driveway which currently provides vehicular access to the adjacent "Timber Tops" residential development. In this regard, the proposed new driveway has been shifted 2m to the north, thereby creating a 2m wide landscaping strip along the southern boundary of the site for improved privacy between the two developments – i.e. the subject development and "Timber Tops".

The proposed new driveway will also provide vehicular access to the "Timber Tops" development by way of a registered easement. The driveway levels have been designed to comply with *AS2890.2* as well as tie into the existing levels in the vicinity of the "Timber Tops" car parking area, approximately 30m from the front boundary, ensuring that vehicular access to the "Timber Tops" residential development is not disrupted.

Furthermore, the main access driveway has been modified to include two splayed driveways separated by a raised kerb and concrete median *within* the property boundary in order to restrict turning movements to left-in/left-out only, generally in accordance with RMS's standard design.

It is pertinent to note that the location of the proposed site access driveway in Longueville Road complies with Council's site specific *DCP 2010*, *Part C*, *Residential Localities – Locality 7 – 266 Longueville Road (Amendment 25 March 2015)* document which requires access to be provided via the existing easement – i.e. via the "Timber Tops" driveway. During the preparation of the site-specific *Draft DCP*, consultation was made with the RMS who provided their support of the driveway location which was then adopted into the amended *DCP*. In addition, the Panel note that any driveway located at the northern end of the Longueville Road site frontage "would result in an increase in the impact on the neighbours to the north, increase the amount of paving on the site and create two access points instead of one".

The servicing and delivery needs of the proposed development are expected to be undertaken by light commercial vehicles such as "white vans" and the like and small trucks up to and including 6.4m long small rigid trucks. All garbage collection services are to be undertaken by private contractors using a small rigid truck. A servicing/delivery area is proposed at the eastern end of the internal driveway/ramp to accommodate these vehicles, such that they will be able to enter and exit the site whilst travelling in a forward direction at all times.

### **Proposed Intersection Upgrade**

Since the lodgement of the original Development Application, Council and the RMS have explored options to upgrade the intersection of River Road West & Longueville Road. The concept signal plans are reproduced in Appendix A and summarised on the following page.

The short-term option includes:

- provision of a designated right turn lane from Longueville Road into River Road
   West; and
- redesign of the northbound approach to better align the through movement and exaggerate the right turn into Longueville Road

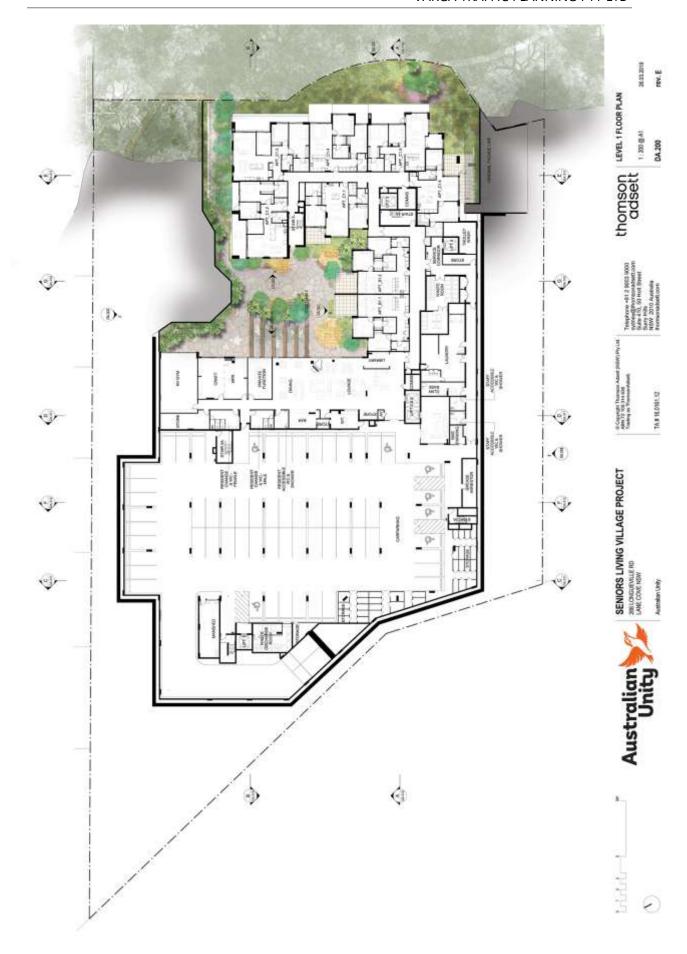
The long-term option includes:

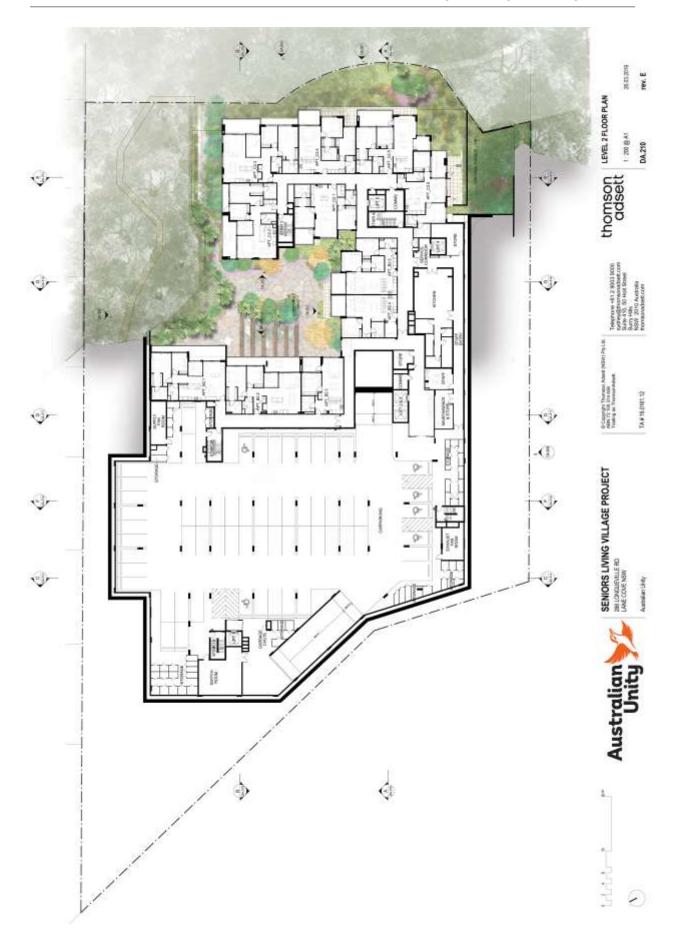
- provision of a designated right turn lane from Longueville Road into River Road
   West; and
- provision of a dedicated right turn lane into Longueville Road and Kenneth Street;
   and
- redesign of the northbound approach to better align the through movement and exaggerate the right turn into Longueville Road.

RMS also note that subject to funding, approval has been granted for the short-term option. Funding for the long-term option is unknown at this stage.

As recommended in the Bitzios peer review traffic study, the future scenario – i.e. existing development plus development traffic – is based on the proposed new short-term intersection layout.

Plans of the proposed amended development have been prepared by *Thomson Adsett Architects* and are reproduced in the following pages.















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### 3. TRAFFIC ASSESSMENT

### **Road Hierarchy**

The road hierarchy allocated to the road network in the vicinity of the site by the Roads and Maritime Services is illustrated on Figure 3.

Epping Road is classified by the RMS as a *State Road* and provides the key east-west road link in the area, linking Epping to Lane Cove. It typically carries two traffic lanes in each direction in the vicinity of the site (including a 24-Hour Bus Lane in both directions), with additional lanes provided at key locations.

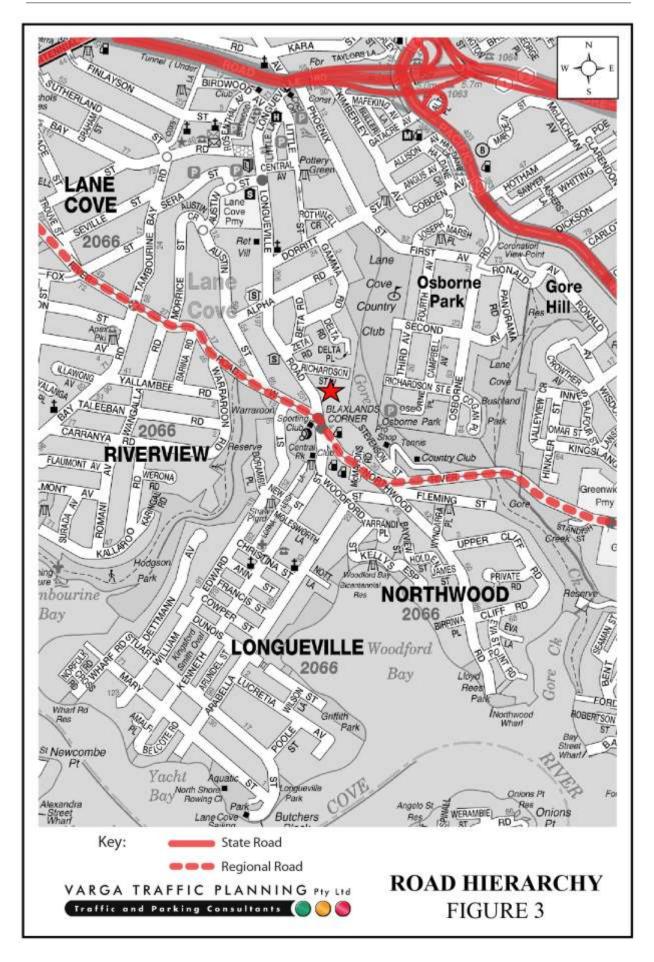
River Road is classified by the RMS as a *Regional Road* and provides the key east-west road link in the area. It typically carries 4 traffic lanes (ie; two lanes in each direction), with some kerbside parking permitted in selected locations only, outside peak periods.

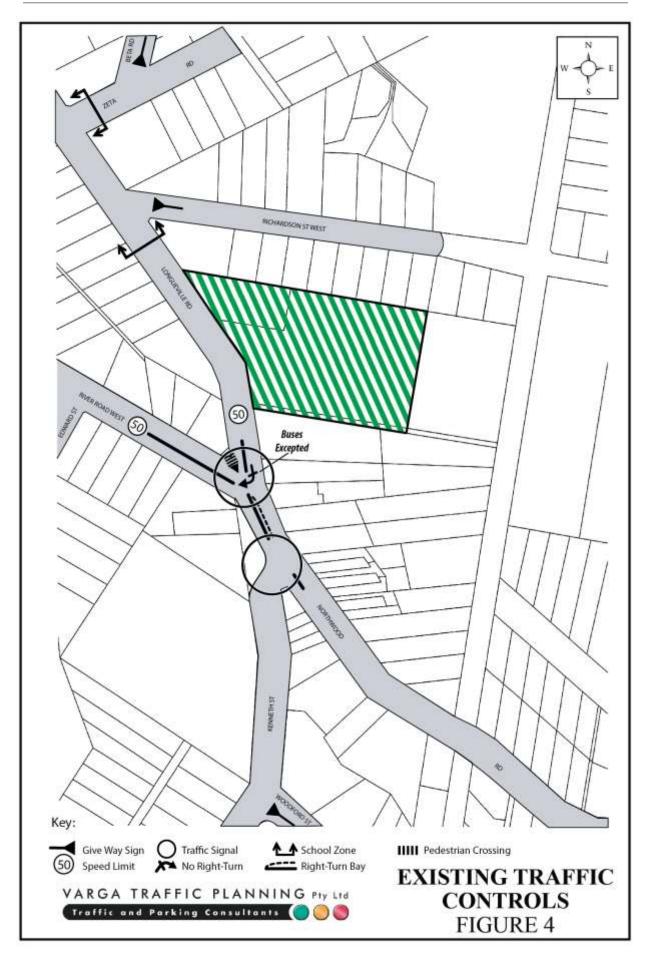
Longueville Road is a local, unclassified road which performs the function of a *Collector Route*. It typically carries one traffic lane in each direction, with kerbside parking generally permitted on both sides of the road.

### **Existing Traffic Controls**

The existing traffic controls which apply to the road network in the vicinity of the site are illustrated on Figure 4. Key features of those traffic controls are:

- a 50 km/h SPEED LIMIT which applies to all roads in the surrounding area
- a 40 km/h SCHOOL ZONE SPEED LIMIT which applies to Longueville Road, within the vicinity of Currambena Primary School and St Michael's Catholic Primary School
- TRAFFIC SIGNALS in River Road where it intersects with Longueville Road
- TRAFFIC SIGNALS in Northwood Road where it intersects with Kenneth Street





 NO RIGHT-TURN (Buses Excepted) restriction for southbound traffic in Longueville Road at its intersection with River Road West.

### **Existing Public Transport Services**

The existing public transport services available to the site are illustrated on Figure 5. There are currently three bus routes travelling along Longueville Rd past the site, comprising regular services to North Sydney and the City, as well as a number of peak hour express services along the Gore Hill Freeway to/from the City.

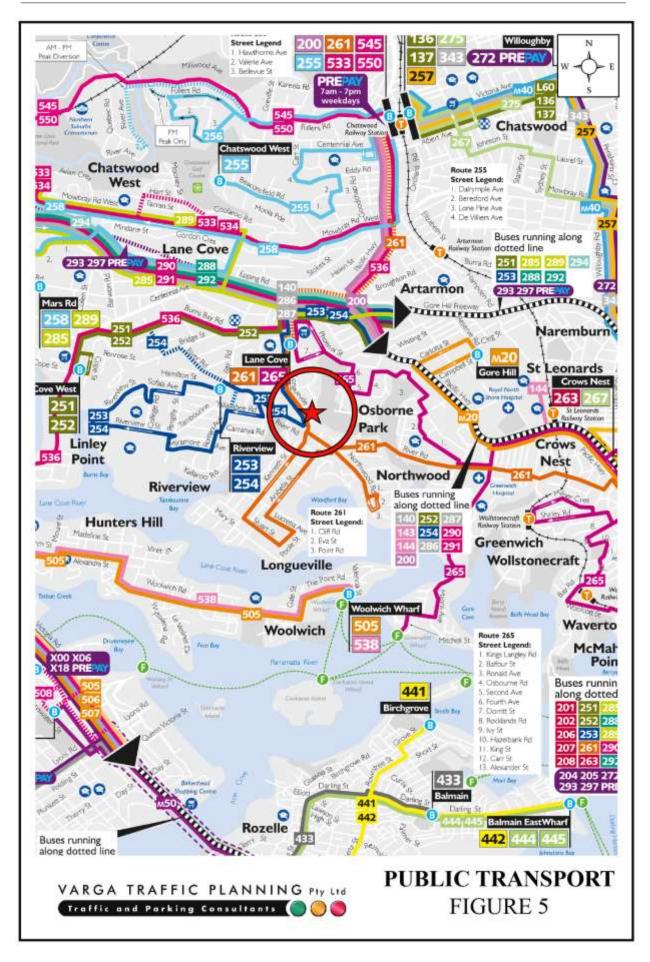
There are more than 110 bus services travelling past the site on weekdays (with a peak hour frequency of 8 services per hour), decreasing to 60 bus services per day on Saturdays and 30 services per day on Sunday and public holidays, as set out in the table below:

	Bus Routes and Frequencies						
Route			Weekdays		Saturday		day
No.	Route	IN	OUT	IN	OUT	IN	OUT
253	Riverview to City (via Fwy)	7	5	-	-	-	-
254	Riverview to City (via Fwy)	29	25	20	19	15	15
261	City King St Wharf to	23	25	11	12	-	-
Chatswood via Longueville							
TOTAL		59	55	31	31	15	15

All of the abovementioned bus services traverse the Lane Cove shops where an extensive range of shops (including supermarkets), restaurants, cafes and services such as banks and the post office are located.

All of the bus services also traverse the Bus Interchange located on Longueville Road (near the intersection of Epping Rd), allowing interchange with other connecting bus services to Epping, Chatswood, St Leonards railway stations and the medical facilities located in the vicinity of RNS Hospital.

In addition, an interchange is also possible on the Pacific Hwy near Gore Hill TAFE college with the intra-regional Metrobus M20 service which operates between Artarmon and Botany via Central Railway seven days per week, with weekday services every 15 minutes (every 10 minutes during the morning and afternoon peak) and weekend services every 20 minutes.



The site is therefore considered to be ready accessible by public transport services. Experience at other *Australian Unity* RACFs indicate that a substantial portion of staff travel to/from work via public transport.

### **Existing Traffic Conditions**

An indication of the existing traffic conditions on the road network in the vicinity of the site is provided by peak period traffic surveys undertaken as part of this revised traffic study. The traffic surveys were undertaken at the Longueville Road & River Road intersection, the Northwood Road & Kenneth Street intersection and also the existing "Timber Tops" site access driveway. The results of the traffic surveys are reproduced in full in Appendix B and reveal that:

- the morning network peak period occurred between 8:00am and 9:00am whilst the afternoon network peak period occurred between 4:45pm and 5:45pm
- two-way traffic flows past the site frontage in Longueville Road on the day of the surveys were in the order of 700 vehicles per hour (vph) during the morning and afternoon peak period, and
- two-way traffic flows using the existing "Timber Tops" site access driveway were 6 vph during the morning peak period and 8 vph during the afternoon peak period.

### **Projected Traffic Generation**

An indication of the traffic generation potential of the proposed development is provided by reference to the Roads and Maritime Services publication *Guide to Traffic Generating Developments, Section 3 – Land Use Traffic Generation (October 2002)* and the updated *Technical Direction TDT 2013/04a.* 

The RMS *Guidelines* and the *TDT 2013/04a* are based on extensive surveys of a wide range of landuses and nominate the following traffic generation rates which are applicable to the development proposal:

#### **Housing Seniors**

0.4 peak hour vehicle trips per dwelling

The RMS *Guidelines* also make the following observation in respect of housing for aged and disabled persons:

#### **Definition**

Residential accommodation which may take any building form which is to be intended to be used permanently as housing for the accommodation of aged or disabled persons. The hostel may consist of residencies or a grouping of 2 or more self-contained dwellings and include facilities such as staff accommodation, chapels, medical rooms, recreation facilities, shops and/or therapy rooms.

#### **Factors**

These figures at the lower end of the above rates concentrate on *subsidised* developments (often run by religious organisations). Generation rates of *resident fun*ded developments are often greater, as indicated at the higher end of the range.

Application of the above traffic generation rate to the 82 ILUs and 70 standard aged care beds outlined in the development proposal yields a traffic generation potential of approximately 61 vehicle trips per hour during commuter peak periods as follows:

PROJECTED TRAFFIC GENERATION				
82 Independent Living Units:	33 vph			
70 Aged Care Beds:	28 vph			
TOTAL TRAFFIC GENERATION POTENTIAL:	61 vph			

In practice, the actual traffic generation potential of the proposed development is likely to be *less* than is set out in the table above, as RACF beds tend to generate *less traffic* activity than ILUs. The RMS *TDT 2013/04a* also makes the following observations:

"Note that morning site peak hour does not generally coincide with the network peak hour".

Furthermore, the RACF site peak will also *not* coincide with the network peak period as the morning shift change will occur *prior to* the morning network peak which is 8:00am and 9:00am whilst the afternoon shift change will occur *prior to* the afternoon network peak which is 4:45pm and 5:45pm.

Peak visitor periods for both the ILUs and RACF will likely be during the middle of the day and on weekends when on-road traffic is much lower than the morning and afternoon peak periods.

In reality, the only component of the proposed development that *might* occur during the network peak periods is the ILUs in the *afternoon* which is 4:45pm and 5:45pm.

Whilst the *Bitzios* peer review traffic study recommended that a *nett traffic generation* using 53 AM trips and 53 PM trips be adopted (i.e. proposed less existing), this recommendation has *not* been implemented in order to provide a more *rigorous* assessment, therefore the above traffic generation potential of 61 vph has continued to be adopted to assess the traffic impacts on the adjacent public road network, as set out below. Furthermore, the SIDRA model also takes into account the RMS request for all turning movements into/out of the site to be restricted to left-in/left-out only.

In any event, that projected increase in traffic activity as a consequence of the development proposal is minimal and will clearly not have any unacceptable traffic implications in terms of road network capacity, as is demonstrated by the following section of this report.

### **Traffic Splits & Trip Distribution**

The table below indicates the traffic splits that have been adopted in the traffic model.

Development	AM Peak Volumes		AM Peak Volumes PM Peak Volumes		Volumes
	In	Out	In	Out	
Subject Site	30 vph	31 vph	31 vph	30 vph	
Timber Tops	0 vph	6 vph	5 vph	3 vph	
TOTAL	30 vph	37 vph	36 vph	33 vph	

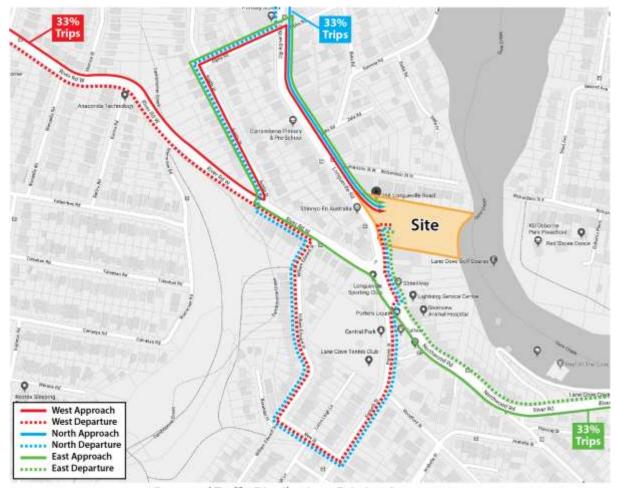
In terms of distribution, it has been assumed that development will approach and depart the site *evenly distributed* between the west via River Road West, the north via Longueville Road and the east via Northwood Road.

It is worth noting that the *Bitzios* peer review traffic study adopted a distribution of 20% to the west via River Road West, 30% to the north via Longueville Road, 40% to the east along Northwood Road and 10% to the south via William Edward Street.

Whilst the distributions adopted in the traffic model differ slightly to the *Bitzios* distributions, in traffic volume terms there are minimal differences between the two.

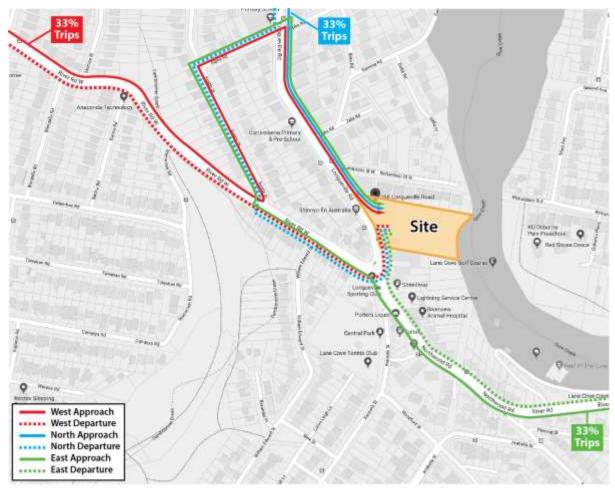
Direction	AM Peak Volumes		PM Peak	Volumes
	In	Out	In	Out
North	10 vph	12 vph	12 vph	11 vph
South	0 vph	0 vph	0 vph	0 vph
East	10 vph	13 vph	12 vph	11 vph
West	10 vph	12 vph	12 vph	11 vph

At present, one option for any vehicle, other than buses, wishing to head west along River Road West after travelling south along Longueville Road would be to cross over Northwood Road onto Kenneth Street, then west onto New Street, north onto William Edward Street and then west along River Road West, as indicated in the figure below. This would also be the case for the proposed development's traffic when exiting the site.



**Proposed Traffic Distribution - Existing Arrangements** 

In accordance with the recommendations in the *Bitzios* peer review traffic study, the traffic model takes into consideration the proposed new short-term upgrade of the River Road West & Longueville Road traffic signals which will permit right turn movements for traffic turning from Longueville Road onto River Road West for *all* traffic, as indicated in the figure below.



Proposed Traffic Distribution - Proposed Arrangements

For the purposes of this assessment it has been assumed that 10% of the existing traffic turning right into Kenneth Street from Northwood Road is performing the abovementioned circuit back to River Road West (i.e. 16 vph AM and 19 vph PM). These volumes have therefore been removed from the relevant turning volumes and added to the proposed new right turn directly onto River Road West.

The proposed new upgrade of the River Road West and Longueville Road traffic signals which will allow right turn movements for *all* vehicles, not just buses, will therefore remove in the order of 40 vph from the Kenneth Street/New Street/William Edward Street circuit.

#### **SIDRA Traffic Model**

The traffic impacts of development proposals primarily concern the effects that the additional traffic flows the development generates may have on the operational performance of the nearby road network. Those effects can be assessed using the SIDRA NETWORK capacity analysis program as is widely used by the RMS and many LGAs for this purpose. Criteria for evaluating the results of SIDRA NETWORK capacity analysis are reproduced in the following pages.

The *Bitzios* peer review traffic study recommended that a number of modifications were required to the traffic model including intersection geometry, lane widths, network geometry, priorities, phasing and timing and also based on the RMS's IDM (Independent Diagnostic Monitor) data. A full copy of the IDM data is reproduced in Appendix C. Screenshots of the intersection layouts of the IDM data are reproduced below.



Source: RMS

### **Traffic Implications - Road Network Capacity**

The results of the revised SIDRA capacity analysis, which take into consideration the *Bitzios* recommendations, are reproduced in full in Appendix D and summarised in the following pages. The SIDRA results of the nearby Longueville Road/River Road intersection are summarised in the following tables.

TABLE 1 – RESULT OF SIDRA ANALYSIS LONGUEVILLE ROAD & RIVER ROAD					
EXISTING PROPOSED				OSED	
	AM	PM	AM	PM	
Level of Service	Е	В	С	В	
Average Vehicle Delays	67.3	16.5	28.8	21.3	
Degree of Saturation	1.030	0.630	0.861	0.711	

TABLE 2 – RESULT OF SIDRA ANALYSIS NORTHWOOD ROAD & KENNETH STREET					
	EXIS	TING	PROP	OSED	
	AM	PM	AM	PM	
Level of Service	В	В	В	В	
Average Vehicle Delays	16.1	23.9	17.3	21.4	
Degree of Saturation	0.922	0.895	0.938	0.866	

TABLE 3– RESULT OF SIDRA ANALYSIS LONGUEVILLE ROAD & SITE ACCESS DRIVEWAY					
	EXIS	TING	PROP	OSED	
	AM	PM	AM	PM	
Level of Service	A	A	A	A	
Average Vehicle Delays	0.0	0.1	0.2	0.2	
Degree of Saturation	0.245	0.197	0.244	0.195	

In summary therefore, the revised capacity analysis reconfirms that the traffic impacts of the development proposal will be *statistically insignificant* and will clearly not have any unacceptable traffic implications in terms of road network capacity.

# Criteria for Interpreting Results of Sidra Analysis

### 1. Level of Service (LOS)

LOS	Traffic Signals and Roundabouts	Give Way and Stop Signs
'A'	Good operation.	Good operation.
'B'	Good with acceptable delays and spare capacity.	Acceptable delays and spare capacity.
'C'	Satisfactory.	Satisfactory but accident study required.
'D'	Operating near capacity.	Near capacity and accident study required.
'E'	At capacity; at signals incidents will cause excessive	At capacity and requires other control mode.
	delays. Roundabouts require other control mode.	
'F'	Unsatisfactory and requires additional capacity.	Unsatisfactory and requires other control mode.

### 2. Average Vehicle Delay (AVD)

The AVD provides a measure of the operational performance of an intersection as indicated on the table below which relates AVD to LOS. The AVD's listed in the table should be taken as a guide only as longer delays could be tolerated in some locations (ie inner city conditions) and on some roads (ie minor side street intersecting with a major arterial route).

Level of Service	Average Delay per Vehicle (secs/veh)	Traffic Signals, Roundabout	Give Way and Stop Signs
A	less than 14	Good operation.	Good operation.
В	15 to 28	Good with acceptable delays and spare capacity.	Acceptable delays and spare capacity.
С	29 to 42	Satisfactory.	Satisfactory but accident study required.
D	43 to 56	Operating near capacity.	Near capacity and accident study required.
Е	57 to 70	At capacity; at signals incidents will cause excessive delays. Roundabouts require other control mode.	At capacity and requires other control mode.

### 3. Degree of Saturation (DS)

The DS is another measure of the operational performance of individual intersections.

For intersections controlled by traffic signals both queue length and delay increase rapidly as DS approaches 1, and it is usual to attempt to keep DS to less than 0.9. Values of DS in the order of 0.7 generally represent satisfactory intersection operation. When DS exceeds 0.9 queues can be anticipated.

For intersections controlled by a roundabout or GIVE WAY or STOP signs, satisfactory intersection operation is indicated by a DS of 0.8 or less.

### 4. PARKING IMPLICATIONS

### **Existing Kerbside Parking Restrictions**

The existing kerbside parking restrictions which apply to the road network in the vicinity of the site are illustrated on Figure 6 and comprise:

- NO STOPPING restrictions in the vicinity of the Longueville Road/River Road West intersection
- BUS ZONES located on both sides of Longueville Road at regular intervals, including directly in front of the site
- NO PARKING restrictions on the eastern side of Longueville Road between the southern boundary of the site and the traffic signals at the River Road West intersection
- UNRESTRICTED KERBSIDE PARKING elsewhere along this section of Longueville Road.

### **Off-Street Parking Provisions**

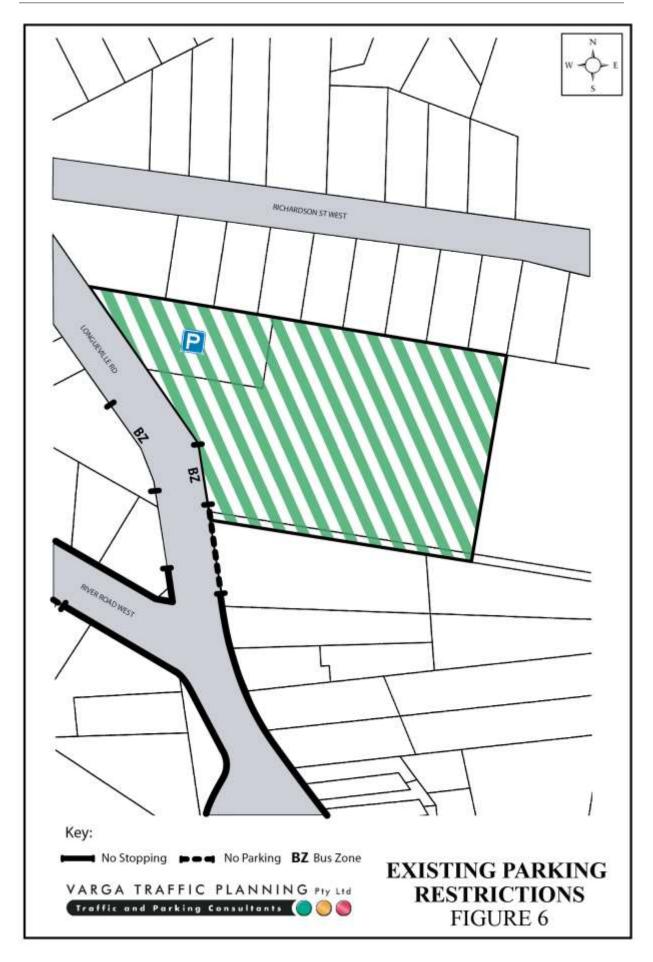
The off-street parking requirements applicable to the development proposal are specified in Council's *DCP 2006* and in the *State Environmental Planning Policy (Housing for Seniors or People with a Disability) 2004* which specify the following off-street car parking requirements applicable to the development proposal:

### **Division 4 Self Contained Dwellings**

#### 50 Standards that cannot be used to refuse development consent for self-contained dwellings

A consent authority must not refuse consent to a development application made pursuant to this Chapter for the carrying out of development for the purpose of a self-contained dwelling (including in-fill self-care housing and services self-care housing) on any of the following grounds:

(d) parking: if at least the following is provided:



- (i) 0.5 parking space for each bedroom where the development application is made by a person other than a social housing provider
- (ii) 1 car space for each 5 dwellings where the development application is made by, or is made by persons jointly with a social housing provider.

#### **Division 2 Residential Care Facilities**

#### 48 Standards that cannot be used to refuse development consent for residential care facilities

A consent authority must not refuse consent to a development application made pursuant to this Chapter for the carrying out of development for the purpose of a residential care facility on any of the following grounds:

- (d) parking for residents and visitors: if at least the following is provided:
  - (i) 1 parking space for each 10 beds in the residential care facility (or 1 parking space for each 15 beds if the facility provides care only for persons with dementia), and
  - (ii) 1 parking space for each 2 persons to be employed in connection with the development and on duty at any one time, and
  - (iii) 1 parking space suitable for an ambulance.

Application of the above parking requirements to the development proposal yields an offstreet car parking requirement of 101 spaces as set out in the table below:

Independent Seniors Living Units: (82 dwellings): 76.5 spaces
Residential Aged Care Visitors (70 beds - standard): 7.0 spaces
Residential Aged Care Staff (max. 34 staff at PM changeover): 17.0 spaces
TOTAL: 100.5 spaces

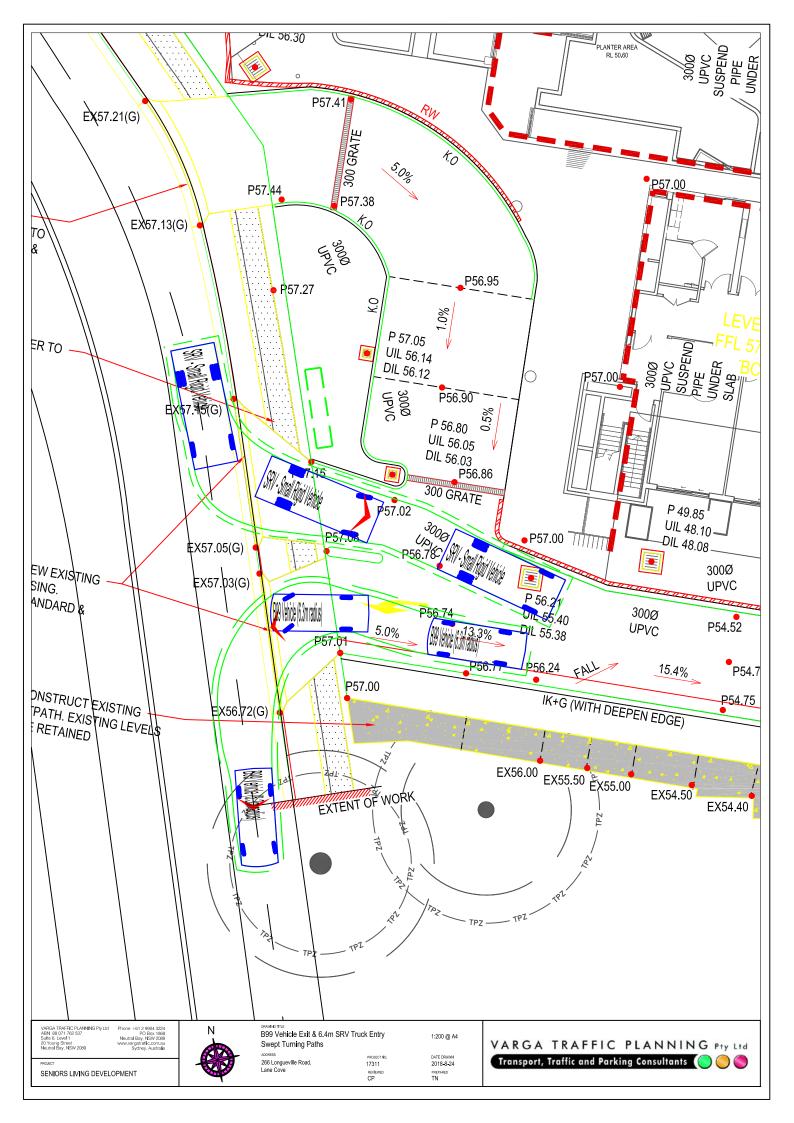
Whilst the *SEPP* does not nominate an off-street parking rate for visitors to the ILUs, visitor parking in the amended scheme is now proposed at a rate of *1 space per 4 units* – i.e. 21 spaces – consistent with the visitor parking rate for residential flat buildings nominated in Council's *DCP*.

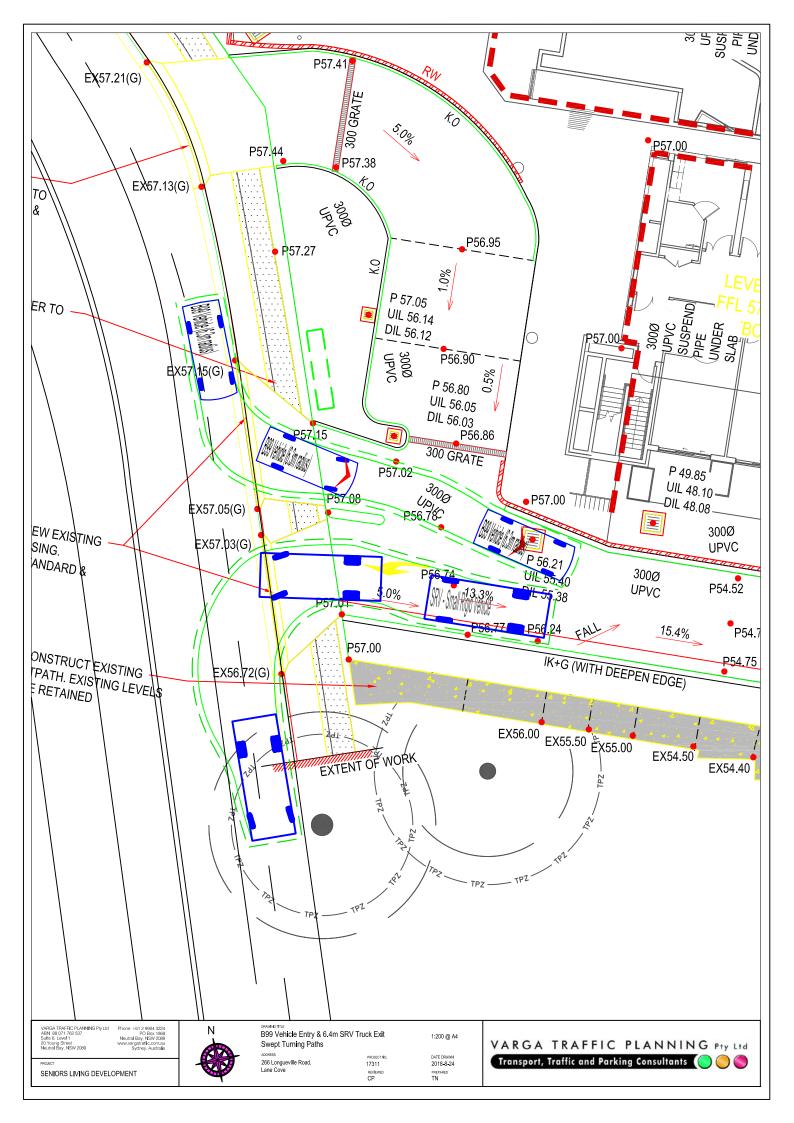
The proposed development makes provision for a total of 122 off-street parking spaces, plus an ambulance bay, thereby satisfying the *SEPP* (and Council's *DCP*) requirements.

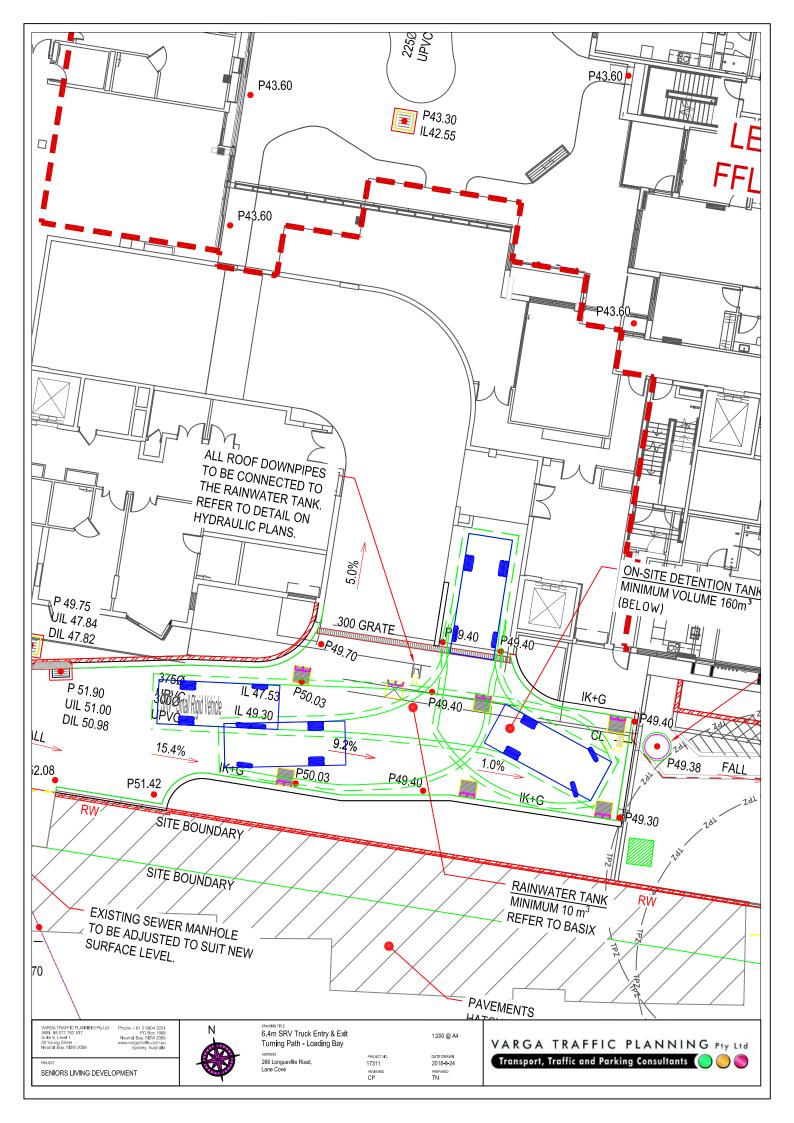
The geometric design layout of the proposed car parking facilities has been designed to comply with the relevant requirements specified in the Standards Australia publication *Parking Facilities Part 1 - Off-Street Car Parking AS2890.1 - 2004* in respect of parking bay dimensions and aisle widths.

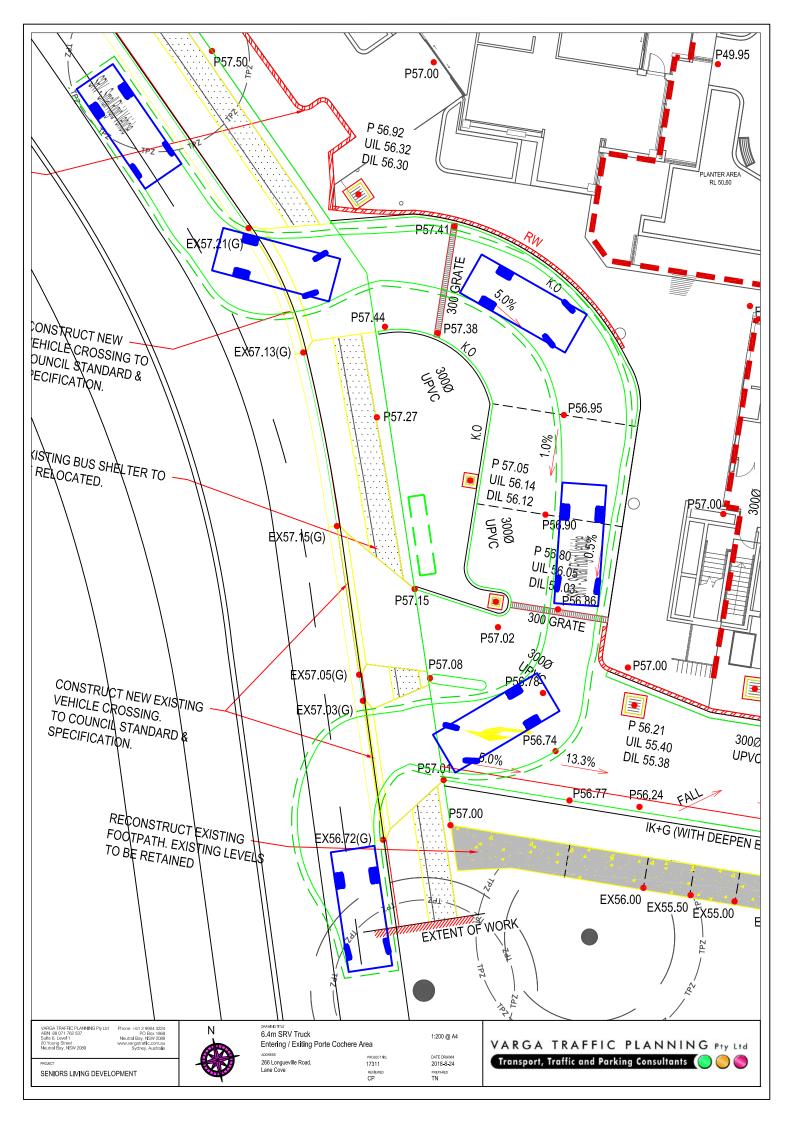
The amended vehicular access arrangements have also been designed to accommodate the largest vehicle expected to service the site which is a 6.4m long SRV truck. A series of swept turning path diagrams have been prepared which demonstrate that SRV trucks, and light passenger vehicles, can enter and exit the site in a forward direction at all times.

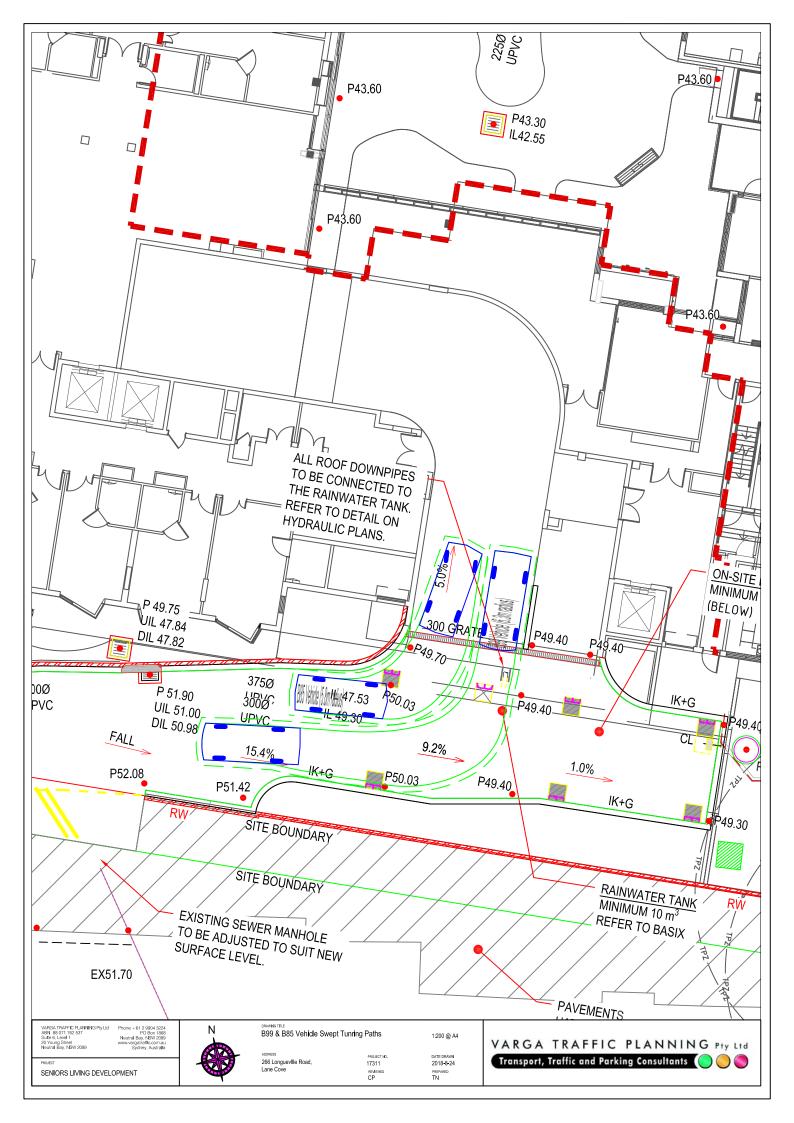
In summary, the proposed parking, loading and access arrangements satisfy the relevant requirements specified in *SEPP* (2004), Council's *DCP*, Australian Standards and by the RMS, and it is therefore concluded that the proposed development will not have any unacceptable parking, servicing or access implications.











## APPENDIX A

CONCEPT TRAFFIC SIGNAL PLAN



10 July 2018

The General Manager Lane Cove Municipal Council PO Box 20 LANE COVE NSW 1595

Attention: Sashika Perera, Coordinator - Traffic and Transport

#### Re: River Road West/Longueville Road Intersection Upgrade

Roads and Maritime Services have reviewed the River Road West/Longueville Road intersection and support Council with its preferred short-term and long-term options.

The short-term option includes:

- Provision of a designated right turn lane from Longueville Road into River Road West; and
- Redesign of the northbound approach to better align the through movement and exaggerate the right turn into Longueville Road.

The long-term option includes:

- Provision of a designated right turn lane from Longueville Road into River Road West; and
- Provision of a dedicated right turn lane into Longueville Road and Kenneth Street; and
- Redesign of the northbound approach to better align the through movement and exaggerate the right turn into Longueville Road.

It is noted that subject to funding, approval has been granted for the implementation of the short-term option.

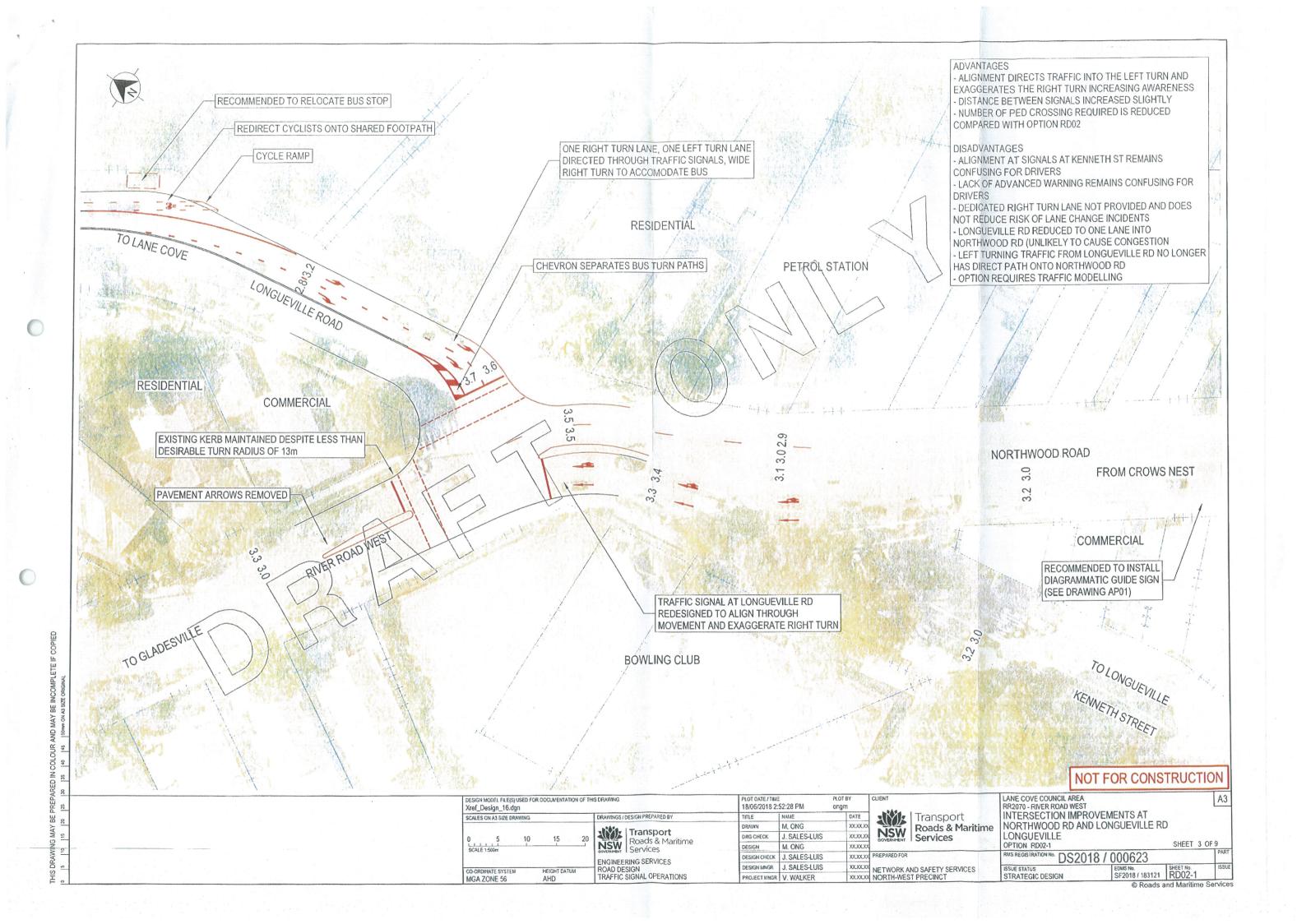
Yours sincerely

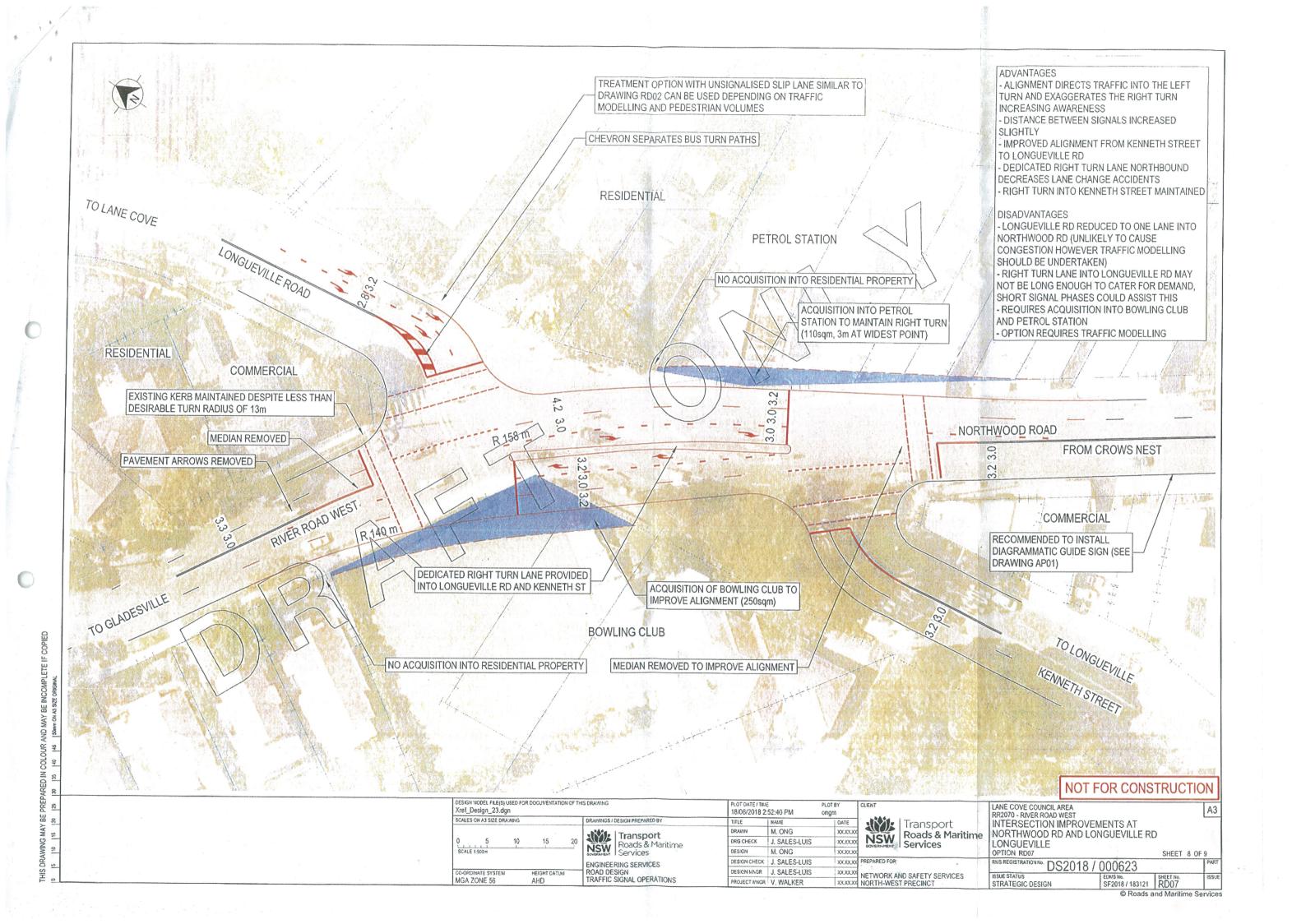
Vicky Walker,

Senior Network and Safety Officer North

Roads and Maritime Services

Roads and Maritime Services

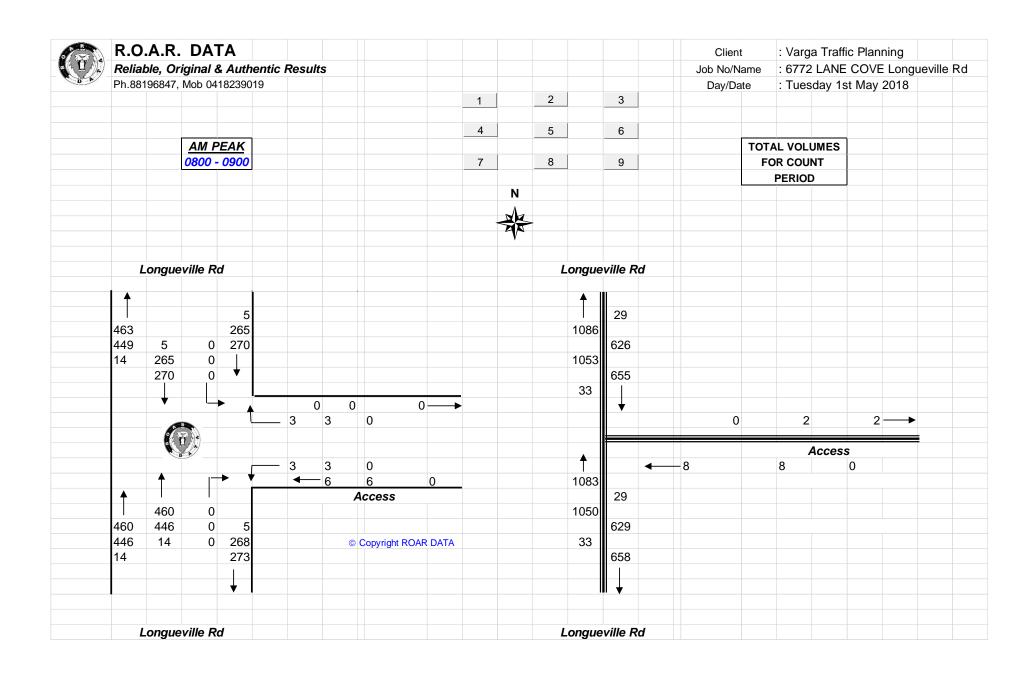




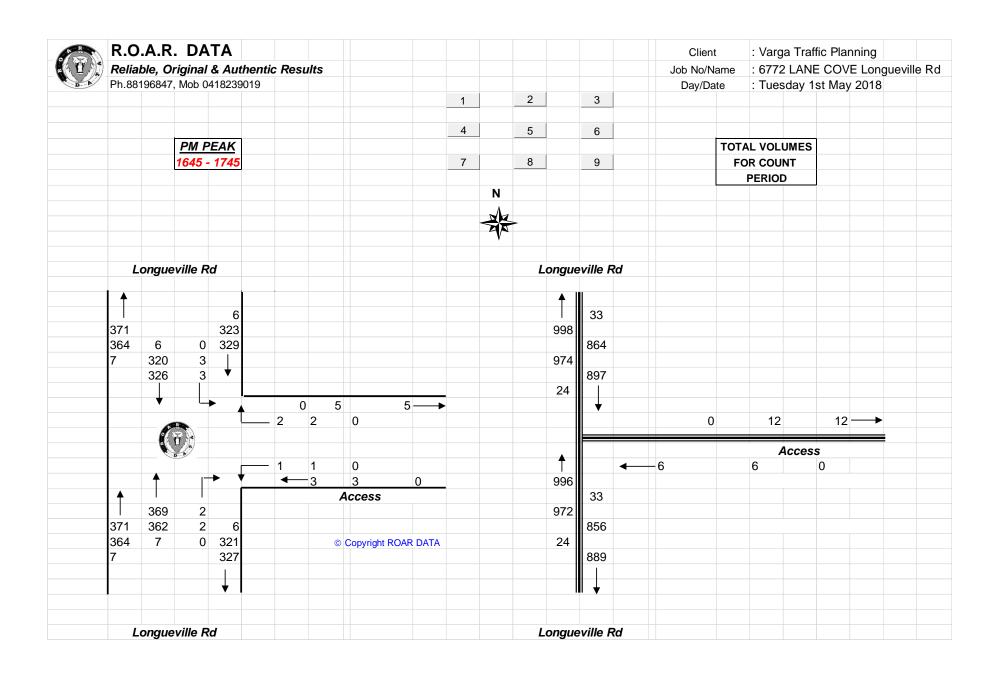
#### APPENDIX B

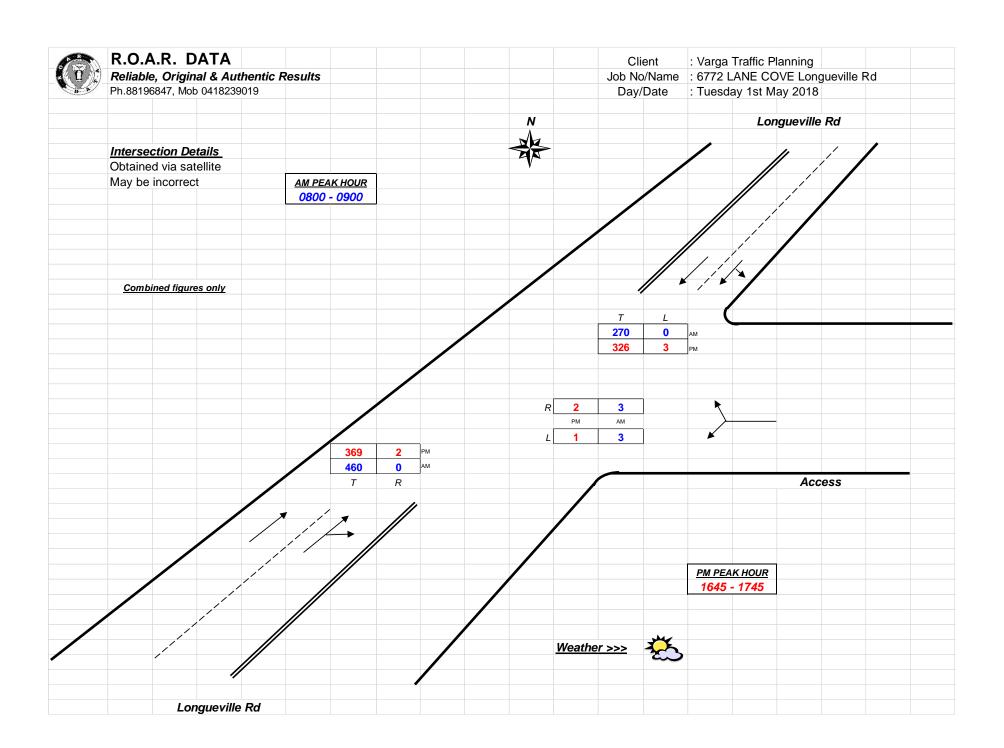
TRAFFIC SURVEY DATA

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0645 - 0700	37	0	1	0	0	74	112	0645 - 0700	4	0	0	0	0	1	5	0645 - 0700	41	0	1	0	0	75	117
0700 - 0715	64	0	0	0	0	65	129	0700 - 0715	5	0	0	0	0	3	8	0700 - 0715	69	0	0	0	0	68	137
0715 - 0730	39	0	0	0	0	102	141	0715 - 0730	5	0	0	0	0	2	7	0715 - 0730	44	0	0	0	0	104	148
0730 - 0745	48	0	0	0	1	82	131	0730 - 0745	2	0	0	0	0	6	8	0730 - 0745	50	0	0	0	1	88	139
0745 - 0800	39	0	1	0	0	93	133	0745 - 0800	1	0	0	0	0	2	3	0745 - 0800	40	0	1	0	0	95	136
0800 - 0815	53	0	1	1	0	86	141	0800 - 0815	1	0	0	0	0	4	5	0800 - 0815	54	0	1	1	0	90	146
0815 - 0830	67	0	1	1	0	123	192	0815 - 0830	2	0	0	0	0	4	6	0815 - 0830	69	0	1	1	0	127	198
0830 - 0845	59	0	1	0	0	139	199	0830 - 0845	1	0	0	0	0	3	4	0830 - 0845	60	0	1	0	0	142	203
0845 - 0900	86	0	0	1	0	98	185	0845 - 0900	1	0	0	0	0	3	4	0845 - 0900	87	0	0	1	0	101	189
0900 - 0915	48	0	0	0	0	82	130	0900 - 0915	3	0	0	0	0	2	5	0900 - 0915	51	0	0	0	0	84	135
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Per Ena	626	U	3	3		1048	1684	Per End	29	U	U	0	U	33	62	Per Ena	655	U	5	3	2	1081	1746
Lights	NO	RTH	E	AST	so	UTH		Heavies	NO	RTH	EA	ST	sou	JTH		Combined	NO	RTH	EA	ST	sou	πH	
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Peak Per	<u>T</u>	<u>L</u>	<u>R</u>	<u>L</u>	<u>R</u>	<u>T</u>	TOT	Peak Per	<u>T</u>	<u> </u>	<u>R</u>	<u>L</u>	<u>R</u>	<u>T</u>	TOT	Peak Per	<u>T</u>	<u> </u>	<u>R</u>	<u>L</u>	<u>R</u>	<u>T</u>	TOT
0630 - 0730	184	0	1	0	0	279	464	0630 - 0730	15	0	0	0	0	8	23	0630 - 0730	199	0	1	0	0	287	487
0645 - 0745 0700 - 0800	188 190	0	1	0	1	323 342	513 534	0645 - 0745 0700 - 0800	16 13	0	0	0	0	12 13	28 26	0645 - 0745 0700 - 0800	204	0	1	0	1	335 355	541 560
0700 - 0800	179	0	2	1	1	363	546	0700 - 0800	9	0	0	0	0	14	26	0700 - 0800	188	0	2	1	1	377	569
	207	0	3	2	1	384	597	0715 - 0815	6	0	0	0	0	16	23	0715 - 0815	213	0	3	2	1	400	619
0730 - 0830 0745 - 0845		0		2	0	441		0730 - 0830	5	0	0	0	t t		18		213	0	4	2	0	454	683
0745 - 0845 0800 - 0900	218	1	3	3	+	441	665 717	0800 - 0900	5	0	0		0	13	19	0745 - 0845		0	3	3	0	460	+
	265	0		2	0	1			7			0	t t	14		0800 - 0900	270				0		736
0815 - 0915 0830 - 0930	260 235	0	1	1	0	442 385	706 623	0815 - 0915 0830 - 0930	8	0	0	0	0	12 9	19 17	0815 - 0915 0830 - 0930	267 243	0	2	1	1	454 394	725 640
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PEAK HR	265	0	3	3	0	446	717	PEAK HR	5	0	0	0	0	14	19	PEAK HR	270	0	3	3	0	460	736

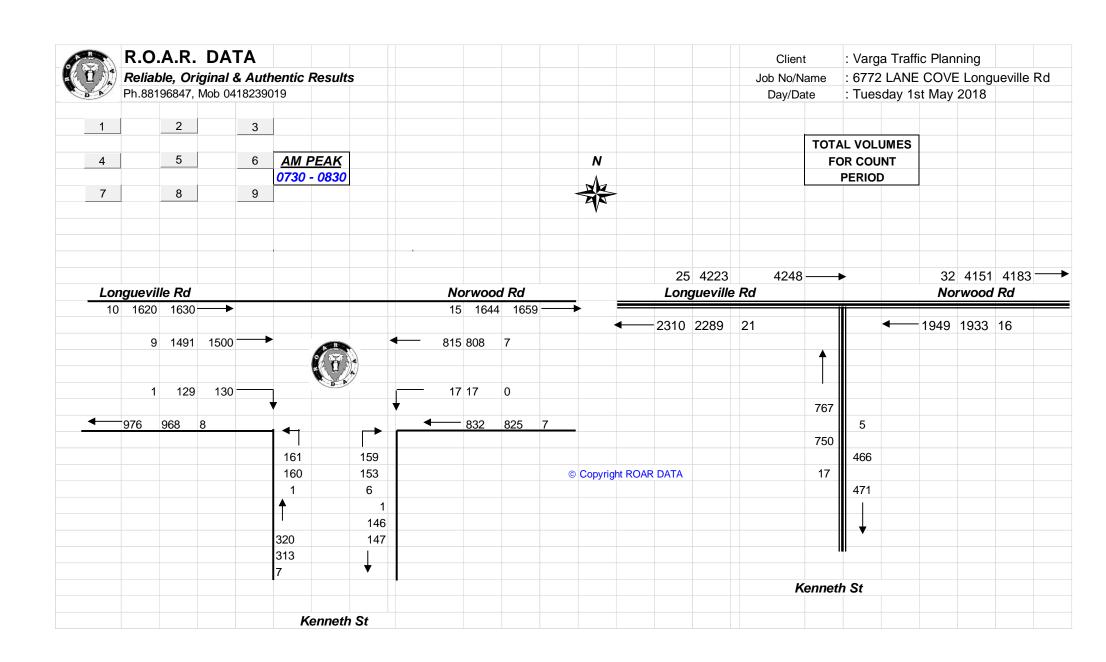


	R.O	.A.R	. DA	ATA																			
	Relia	ble, Or	rigina	l & Au	thentic	Resu	ılts	PEDS	NOR.	H	EA	ST	sol	JTH		PEDS	NO	RTH	E/	ST	so	UTH	
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1545 - 1600	59	0	1	0	0	68	128	1545 - 1600	7	0	0	0	0	5	12	1545 - 1600	66	0	1	0	0	73	140
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1815 - 1830	67	1	1	0	0	76	145	1815 - 1830	3	0	0	0	0	1	4	1815 - 1830	70	1	1	0	0	77	149
Per End	854	10	4	2	2	970	1842	Per End	33	0	0	0	0	24	57	Per End	887	10	4	2	2	994	1899
<u>Lights</u>	NO	RTH	E/	AST	so	UTH		<u>Heavies</u>	NOR'	TH	EA	ST	sol	JTH		Combined	NO	RTH	EA	AST	so	UTH	
	_	ueville Rd	Aco	cess		ieville Rd			Longue Rd		Acc	ess	Longu R	ieville d			Longu R	ueville Rd	Acc	ess		ueville Rd	
Peak Per	Т	L	R	L	R	Т	тот	Peak Per	Т	L	R	L	R	Т	тот	Peak Per	Т	L	R	L	R	Т	тот
1530 - 1630	248	2	1	0	0	296	547	1530 - 1630	19	0	0	0	0	11	30	1530 - 1630	267	2	1	0	0	307	577
1545 - 1645	261	2	1	1	0	288	553	1545 - 1645	15	0	0	0	0	13	28	1545 - 1645	276	2	1	1	0	301	581
1600 - 1700	282	2	0	1	0	319	604	1600 - 1700	10	0	0	0	0	10	20	1600 - 1700	292	2	0	1	0	329	624
1615 - 1715		1	0	1	0	342	649	1615 - 1715	8	0	0	0	0	9	17	1615 - 1715		1	0	1	0	351	666
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1645 - 1745	320	3	2	1	2	362	690	1645 - 1745	6	0	0	0	0	7	13	1645 - 1745	326	3	2	1	2	369	703
1700 - 1800	303	4	2	1	2	345	657	1700 - 1800	8	0	0	0	0	7	15	1700 - 1800	311	4	2	1	2	352	672
1715 - 1815	290	7	2	1	2	329	631	1715 - 1815	8	0	0	0	0	7	15	1715 - 1815	298	7	2	1	2	336	646
1730 - 1830	298	8	2	0	1	320	629	1730 - 1830	9	0	0	0	0	5	14	1730 - 1830	307	8	2	0	1	325	643
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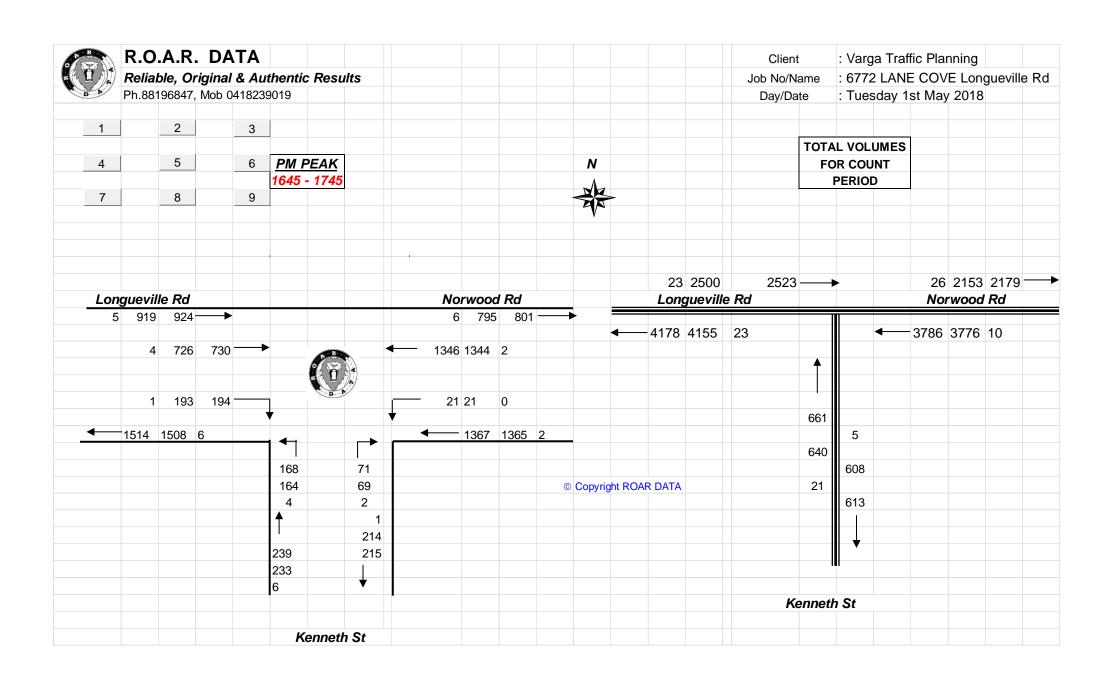


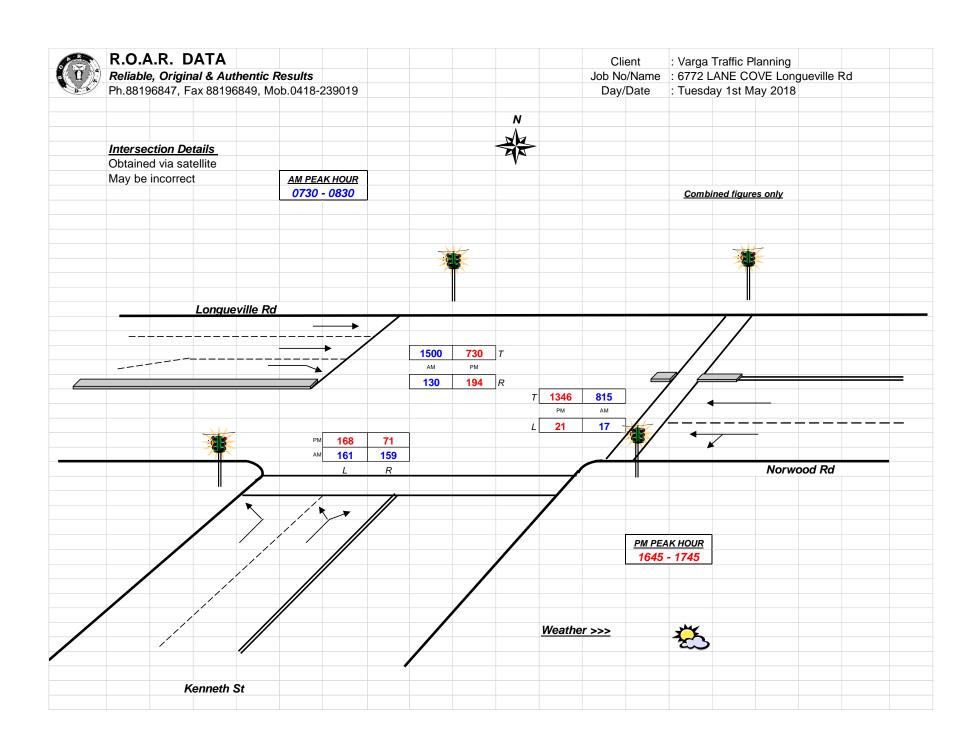


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Job No/N	ame	: 6772	LANE	COVE	E Long	ueville	Rd	0715 - 0730		0	;	3		5	8	0715 - 0815	1	1	1	5	1	7	33
Day/Da	ite	: Tues	day 1s	st May	2018			0730 - 0745		1		5	:	2	8	0730 - 0830	1	1	1	4	1	2	27
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0645 - 0700	276	32	32	16	7	131	494	0645 - 0700	2	0	0	1	0	0	3	0645 - 0700	278	32	32	17	7	131	497
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0815 - 0830	368	35	48	35	6	205	697	0815 - 0830	1	1	0	0	0	0	2	0815 - 0830	369	36	48	35	6	205	699
0830 - 0845	335	38	41	18	4	166	602	0830 - 0845	0	0	1	1	1	2	5	0830 - 0845	335	38	42	19	5	168	607
0845 - 0900	370	50	38	17	8	163	646	0845 - 0900	1	0	0	0	0	3	4	0845 - 0900	371	50	38	17	8	166	650
0900 - 0915	277	33	32	16	2	150	510	0900 - 0915	1	1	1	1	0	0	4	0900 - 0915	278	34	33	17	2	150	514
0915 - 0930	273	37	29	20	8	124	491	0915 - 0930	1	0	1	0	0	1	3	0915 - 0930	274	37	30	20	8	125	494
Per End	3818	405	417	333	61	1872	6906	Per End	22	3	7	10	2	14	58	Per End	3840	408	424	343	63	1886	6964
Lights	WE	ST	so	UTH	EA	ST		Heavies	WE	ST	so	UTH	EA	ST		Combined	WE	ST	so	UTH	EA	ST	1
	Longu R		Kenn	eth St	Norwo	od Rd			_	ueville Rd	Kenn	eth St	Norwo	od Rd			Longu R		Kenn	eth St	Norwo	od Rd	
Peak Per	Т	R	L	R	L	Т	тот	Peak Per	Т	R	L	R	L	Т	тот	Peak Per	Т	R	L	R	L	Т	тот
0630 - 0730	1072	118	117	109	22	461	1899	0630 - 0730	10	1	3	2	1	1	18	0630 - 0730	1082	119	120	111	23	462	1917
0645 - 0745	1251	125	139	125	20	593	2253	0645 - 0745	15	1	3	5	1	2	27	0645 - 0745	1266	126	142	130	21	595	2280
0700 - 0800	1319	117	146	160	17	636	2395	0700 - 0800	15	1	3	6	1	7	33	0700 - 0800	1334	118	149	166	18	643	2428
0715 - 0815	1399	119	145	159	15	732	2569	0715 - 0815	13	1	1	6	0	8	29	0715 - 0815	1412	120	146	165	15	740	2598
0730 - 0830	1491	129	160	153	17	808	2758	0730 - 0830	9	1	1	6	0	7	24	0730 - 0830	1500	130	161	159	17	815	2782
0745 - 0845	1445	134	163	136	18	766	2662	0745 - 0845	3	1	2	4	1	8	19	0745 - 0845	1448	135	165	140	19	774	2681
0800 - 0900	1471	160	162	102	22	755	2672	0800 - 0900	2	1	2	2	1	6	14	0800 - 0900	1473	161	164	104	23	761	2686
0815 - 0915	1350	156	159	86	20	684	2455	0815 - 0915	3	2	2	2	1	5	15	0815 - 0915	1353	158	161	88	21	689	2470
0830 - 0930	1255	158	140	71	22	603	2249	0830 - 0930	3	1	3	2	1	6	16	0830 - 0930	1258	159	143	73	23	609	2265
PEAK HR	1491	129	160	153	17	808	2758	PEAK HR	9	1	1	6	0	7	24	PEAK HR	1500	130	161	159	17	815	2782

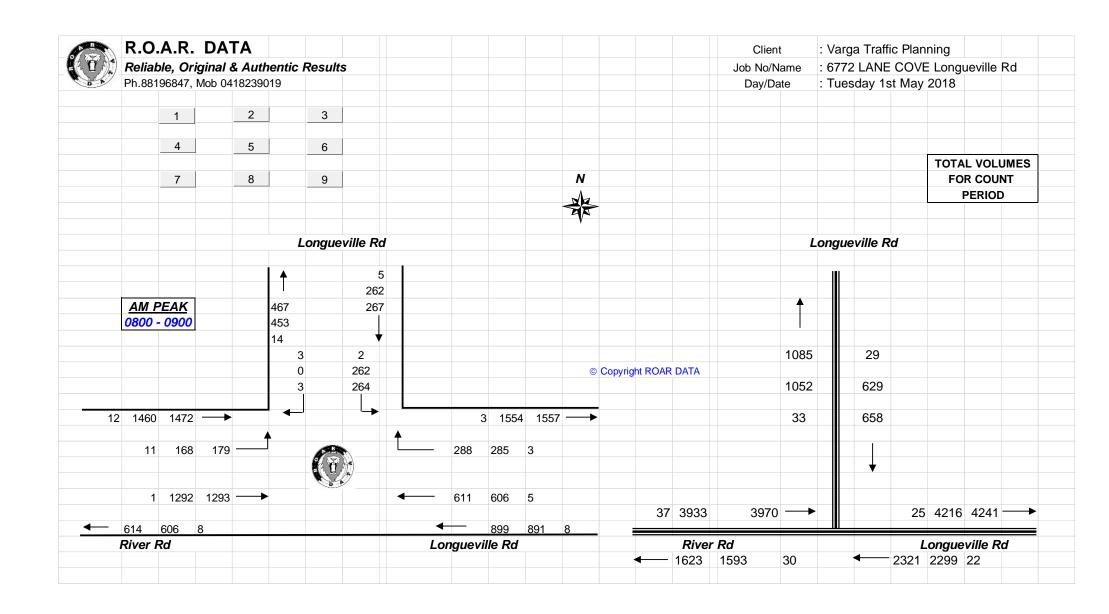


4	R.O	.A.R	. DA	ΤA																			
		<b>ble, O</b> i 196847.	_			c Resu	ılts	PEDS	Longu			UTH eth St	EAS <sup>*</sup>			<u>PEDS</u>	Longi	EST ueville		UTH eth St		AST ood Rd	
	1 11.00	130047	, IVIOD C	41023	3013			Time Per		?d					тот	Peak Per		Rd					TOT
								1530 - 1545		0		0	4		4	1530 - 1630		0		5		35	50
01.		. \ /		"- DI-				1545 - 1600		0		2	13		15	1545 - 1645		0		4		39	63
Client			a Traf					1600 - 1615		0		5	14		19	1600 - 1700		0		2		33	55
Job No/Na						guevill	е ка	1615 - 1630		0		8	4		12	1615 - 1715		0		7		23	40
Day/Da	te	: Tues	sday 1	st May	y 2018	3		1630 - 1645		0		9	8		17	1630 - 1730		0		1		31	42
								1645 - 1700		0		0	7		7	1645 - 1745		0		2		28	30
								1700 - 1715		0		0	4		4	1700 - 1800		0	;	-		29	32
								1715 - 1730		0		2	12		14	1715 - 1815		0		4		32	36
								1730 - 1745		0		0	5		5	1730 - 1830		0		2	2	24	26
								1745 - 1800		0		1	8		9								
								1800 - 1815		0		1	7		8	PEAK HR	(	0	2	2	2	28	30
								1815 - 1830		0		0	4		4								
								Per End	(	)	2	28	90		118								
Lights	\A/E	EST	80	JTH	E/	AST		Heavies	\A/E	EST	80	UTH	EAS.	-		Combined	\A/E	EST	801	UTH	E/	AST	
Ligitis		ieville						neavies	Longu							Combined		ueville					
	R	?d	Kenn	eth St		ood Rd			R	?d	Kenn	eth St	Norwoo				F	Rd	Kenn	eth St	Norwo	ood Rd	
Time Per	<u>T</u>	<u>R</u>	<u>L</u>	<u>R</u>	<u>L</u>	I	TOT	Time Per	<u>I</u>	<u>R</u>	L	<u>R</u>	<u>L</u>	<u>T</u>	TOT	Time Per	I	<u>R</u>	L	<u>R</u>	L	<u>T</u>	TOT
1530 - 1545	153	38	46	17	9	255	518	1530 - 1545	1	0	1	0	0	1	3	1530 - 1545	154	38	47	17	9	256	521
1545 - 1600	138	38	25	13	6	316	536	1545 - 1600	1	0	2	1	1	2	7	1545 - 1600	139	38	27	14	7	318	543
1600 - 1615	116	41	33	10	5	289	494	1600 - 1615	5	1	2	1	0	1	11	1600 - 1615	121	42	35	11	5	291	505
1615 - 1630 1630 - 1645	149 154	32 52	43 34	11 14	4 11	307 303	546 568	1615 - 1630 1630 - 1645	2	0	0	0	0	1	8	1615 - 1630 1630 - 1645	151 156	34 52	46 34	11 15	4 11	308 304	554 572
1645 - 1700	181	51	41	18	4	357	652	1645 - 1700	1	0	2	0	0	2	5	1645 - 1700	182	51	43	18	4	359	657
1700 - 1715	156	49	42	16	5	283	551	1700 - 1715	0	0	1	1	0	0	2	1700 - 1715	156	49	43	17	5	283	553
1715 - 1730	195	49	39	14	7	363	667	1715 - 1730	2	0	1	0	0	0	3	1715 - 1730	197	49	40	14	7	363	670
1730 - 1745	194	44	42	21	5	341	647	1730 - 1745	1	1	0	1	0	0	3	1730 - 1745	195	45	42	22	5	341	650
1745 - 1800	179	44	39	21	7	347	637	1745 - 1800	1	0	1	0	0	0	2	1745 - 1800	180	44	40	21	7	347	639
1800 - 1815	172	43	40	13	7	271	546	1800 - 1815	2	0	0	1	0	0	3	1800 - 1815	174	43	40	14	7	271	549
1815 - 1830	185	47	35	13	10	264	554	1815 - 1830	1	0	1	1	0	0	3	1815 - 1830	186	47	36	14	10	264	557
Per End	1972	528	459	181	80	3696	6916	Per End	19	4	14	7	1	9	54	Per End	1991	532	473	188	81	3705	6970
Lights	WE	ST	so	JTH	E/	AST		Heavies	WE	EST	so	UTH	EAS	Г		Combined	WE	EST	sol	UTH	E/	AST	
		ieville d	Kenn	eth St	Norwo	ood Rd			Long:	ueville Rd	Kenn	eth St	Norwood	l Rd				ueville Rd	Kenn	eth St	Norwo	ood Rd	
Peak Per	т	R	L	R	L	Т	тот	Peak Per	т	R	L	R	L	т	тот	Peak Per	т	R	L	R	L	Т	тот
1530 - 1630	556	149	147	51	24	1167	2094	1530 - 1630	9	3	8	2	1	6	29	1530 - 1630	565	152	155	53	<u>=</u> 25	1173	2123
1545 - 1645	557	163	135	48	26	1215	2144	1545 - 1645	10	3	7	3	1	6	30	1545 - 1645	567	166	142	51	27	1221	2174
1600 - 1700	600	176	151	53	24	1256	2260	1600 - 1700	10	3	7	2	0	6	28	1600 - 1700	610	179	158	55	24	1262	2288
1615 - 1715	640	184	160	59	24	1250	2317	1615 - 1715	5	2	6	2	0	4	19	1615 - 1715	645	186	166	61	24	1254	2336
1010 - 1110	040	201	156	62	27		2438	1630 - 1730	5	0	4	2	0	3	14	1630 - 1730	691	201		64	27	1309	2452
1630 - 1720	686	- ZUI	100	02	1	1306		1630 - 1730 1645 - 1745	4		4				-				160		21	1346	
1630 - 1730	686		161	60	1 21	1 12/1/	2517 '		4	1	4	2	0	2	13	1645 - 1745	730	194	168	71	. ZI	1.340	2530
1645 - 1745	726	193	164	69	21	1344	2517			4	c	2	0	^	40	1700 1000	700	107	165	74			2542
<b>1645 - 1745</b> 1700 - 1800	726 724	193 186	162	72	24	1334	2502	1700 - 1800	4	1	3	2	0	0	10	1700 - 1800	728	187	165	74	24	1334	2512
<b>1645 - 1745</b> 1700 - 1800 1715 - 1815	726 724 740	193 186 180	162 160	72 69	24 26	1334 1322	2502 2497	1700 - 1800 1715 - 1815	4 6	1	2	2	0	0	11	1715 - 1815	746	181	162	71	24 26	1334 1322	2508
<b>1645 - 1745</b> 1700 - 1800	726 724	193 186	162	72	24	1334	2502	1700 - 1800	4												24	1334	

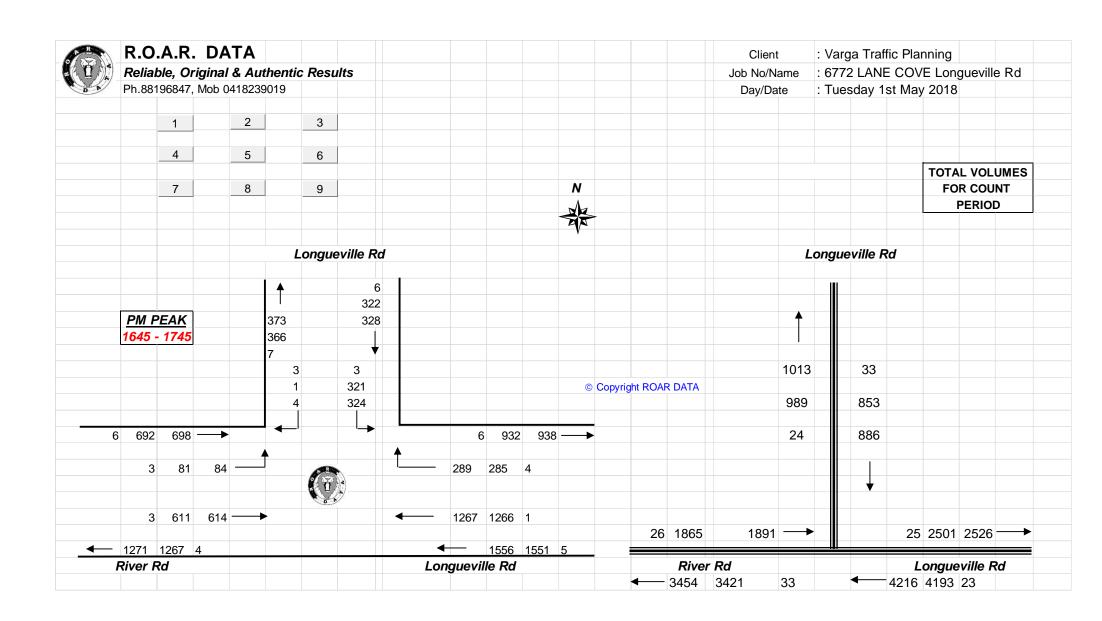


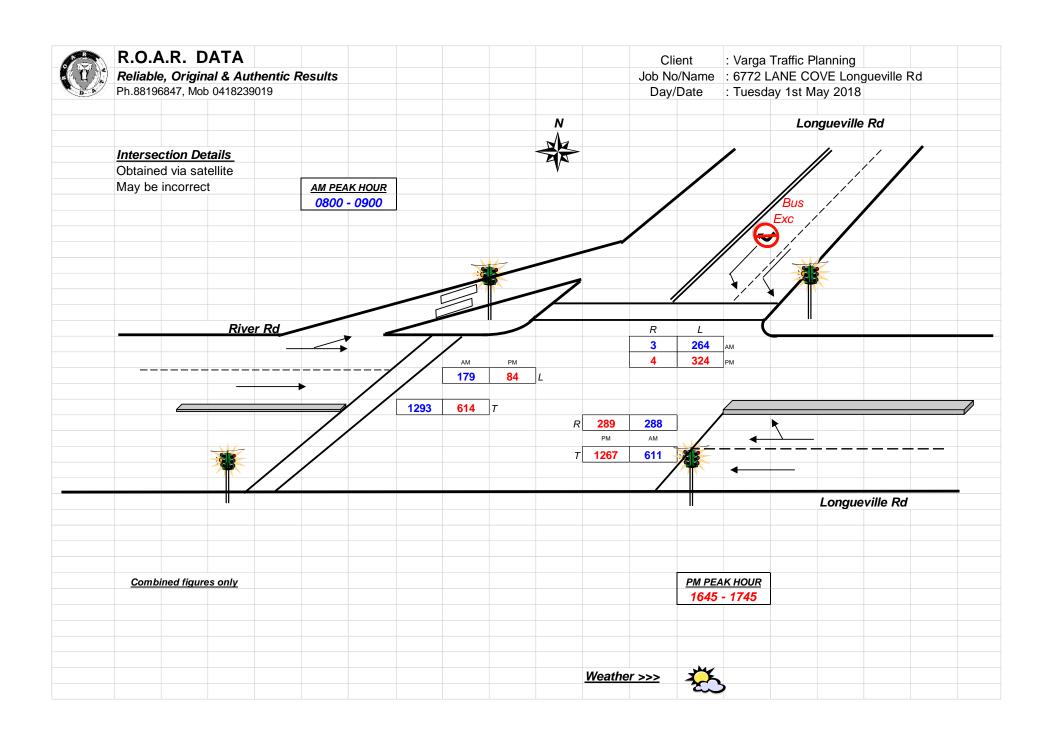


	R.O.	A.R.	DA	TA																			
	Reliab	le, Or	iginal	& Auth	nentic l	Result	s	PEDS	WI	EST		RTH		ST		PEDS	WE	ST		RTH		ST	
D	Ph.881	96847,	Mob 04	1182390	)19			Time Per	Rive	er Rd		ueville Rd		ieville Rd	тот	Peak Per	Rive	r Rd	Longu	ieville d		ieville Rd	тот
								0630 - 0645		3		0		0	3	0630 - 0730	1	4		3		0	17
								0645 - 0700		3		2	(	0	5	0645 - 0745	2	:1	;	3	(	0	24
Client	t	: Varg	a Traff	ic Plan	ning			0700 - 0715		3		0	(	0	3	0700 - 0800	2	:1	į	5	(	0	26
Job No/Na	ame	: 6772	LANE	COVE	E Longi	ueville	Rd	0715 - 0730		5		1	(	0	6	0715 - 0815		:0		3	(	0	28
Day/Da	ite	: Tues	day 1s	st May	2018			0730 - 0745		10		0		0	10	0730 - 0830		3	1			0	30
								0745 - 0800		3		4		0	7	0745 - 0845	1			3		0	25
								0800 - 0815		2		3		0	5	0800 - 0900	1		1	5		0	23
								0815 - 0830		8		0		0	8	0815 - 0915	2			3		0	23
								0830 - 0845		4		1	(	-	5	0830 - 0930	1	4		5	(	0	19
								0845 - 0900		3		2		0	5	DE AIC LID		_					22
								0900 - 0915		5		2		0 0	5	PEAK HR	1	7		3	(	0	23
								0915 - 0930 <b>Per End</b>		2 5 <b>1</b>		<u>∠</u> I5		<u>)</u>	4 66								
								Per Ella		) I	<u> </u>	13	,		00								
Lights	WE	ST	NO	RTH	EA	ST		Heavies	WI	EST	NO	RTH	EA	ST		Combined	WE	ST	NOI	RTH	EA	ST	
-	Rive	r Rd	_	ueville Rd		ieville Rd			Rive	er Rd		ueville Rd	Longu R	ueville Pd			Rive	r Rd	Longu	ieville d		ueville Rd	
Time Per	Т	L	R	L	R	Т	тот	Time Per	Т	L	R	L	R	Т	тот	Time Per	Т	L	R	L	R	Т	ТОТ
0630 - 0645	240	10	0	43	29	<u>-</u> 87	409	0630 - 0645	0	2	0	1	0	0	3	0630 - 0645	240	12	0	44	29	87	412
0645 - 0700	271	15	0	36	61	90	473	0645 - 0700	0	1	1	3	0	0	5	0645 - 0700	271	16	1	39	61	90	478
0700 - 0715	285	19	0	61	41	116	522	0700 - 0715	0	2	4	1	1	3	11	0700 - 0715	285	21	4	62	42	119	533
0715 - 0730	303	34	0	38	66	132	573	0715 - 0730	4	2	2	3	0	1	12	0715 - 0730	307	36	2	41	66	133	585
0730 - 0745	346	21	0	46	62	157	632	0730 - 0745	3	5	0	2	1	2	13	0730 - 0745	349	26	0	48	63	159	645
0745 - 0800	345	27	0	38	66	151	627	0745 - 0800	1	2	0	1	0	3	7	0745 - 0800	346	29	0	39	66	154	634
0800 - 0815	327	34	0	51	52	165	629	0800 - 0815	0	3	1	0	1	1	6	0800 - 0815	327	37	1	51	53	166	635
0815 - 0830	317	33	0	68	82	174	674	0815 - 0830	1	3	1	1	0	1	7	0815 - 0830	318	36	1	69	82	175	681
0830 - 0845	335	60	0	55	87	140	677	0830 - 0845	0	3	1	0	1	2	7	0830 - 0845	335	63	1	55	88	142	684
0845 - 0900	313	41	0	88	64	127	633	0845 - 0900	0	2	0	1	1	1	5	0845 - 0900	313	43	0	89	65	128	638
0900 - 0915	261	31	0	60	51	142	545	0900 - 0915	1	1	2	1	1	2	8	0900 - 0915	262	32	2	61	52	144	553
0915 - 0930 <b>Per End</b>	244 <b>3587</b>	21 <b>346</b>	0 <b>0</b>	45 <b>629</b>	45 <b>706</b>	112 <b>1593</b>	467 6861	0915 - 0930 <b>Per End</b>	0 <b>10</b>	1 27	2 14	1 15	0 <b>6</b>	16	4 88	0915 - 0930 <b>Per End</b>	244 <b>3597</b>	22 <b>373</b>	2 14	46 <b>644</b>	45 <b>712</b>	112 <b>1609</b>	471 6949
								7 07 2110										0.0					1 00.0
<u>Lights</u>	WE	ST		RTH		ST		<u>Heavies</u>	WI	EST		RTH		ST		Combined	WE	ST		RTH		ST	<u> </u>
	Rive	r Rd		ueville Rd		ieville 2d			Rive	er Rd		ueville Rd		ieville Rd			Rive	r Rd	Longu	d d		ueville Rd	
Peak Per	Т	L	R	L	R	Т	тот	Peak Per	Т	L	R	L	R	Т	тот	Peak Per	Т	L	R	L	R	Т	тот
0630 - 0730	1099	78	0	178	197	425	1977	0630 - 0730	4	7	7	8	1	4	31	0630 - 0730	1103	85	7	186	198	429	2008
0645 - 0745	1205	89	0	181	230	495	2200	0645 - 0745	7	10	7	9	2	6	41	0645 - 0745	1212	99	7	190	232	501	2241
0700 - 0800	1279	101	0	183	235	556	2354	0700 - 0800	8	11	6	7	2	9	43	0700 - 0800	1287	112	6	190	237	565	2397
0715 - 0815	1321	116	0	173	246	605	2461	0715 - 0815	8	12	3	6	2	7	38	0715 - 0815	1329	128	3	179	248	612	2499
0730 - 0830	1335	115	0	203	262	647	2562	0730 - 0830	5	13	2	4	2	7	33	0730 - 0830	1340	128	2	207	264	654	2595
0745 - 0845	1324	154	0	212	287	630	2607	0745 - 0845	2	11	3	2	2	7	27	0745 - 0845	1326	165	3	214	289	637	2634
0800 - 0900	1292	168	0	262	285	606	2613	0800 - 0900	1	11	3	2	3	5	25	0800 - 0900	1293	179	3	264	288	611	2638
0815 - 0915	1226	165	0	271	284	583	2529	0815 - 0915	2	9	4	3	3	6	27	0815 - 0915	1228	174	4	274	287	589	2556
0830 - 0930	1153	153	0	248	247	521	2322	0830 - 0930	1	7	5	3	3	5	24	0830 - 0930	1154	160	5	251	250	526	2346
PEAK HR	1292	168	0	262	285	606	2613	PEAK HR	1	11	3	2	3	5	25	PEAK HR	1293	179	3	264	288	611	2638



	R.O	.A.R	. DA	ATA																			
	Relial	ble, Oı	rigina	I & Au	thentic	Resu	ılts	PEDS	WI	EST	NO	RTH	EA	ST		PEDS	WE	ST	NO	RTH	EA	ST	1
W 10			_	0418239				Time Per	Rive	er Rd		ueville Rd	Longu R	ieville d	тот	Peak Per	Rive	r Rd	_	ieville d	_	ieville 2d	тот
								1530 - 1545		8		1		)	9	1530 - 1630	1	9		2		0	21
								1545 - 1600		6		0	(	)	6	1545 - 1645	1	4		1	(	0	15
Client		: Varg	a Tra	ffic Pla	nning			1600 - 1615		2		1	(	)	3	1600 - 1700	1	1		1	(	0	12
Job No/Na	ame	: 6772	LAN	E COV	/E Lon	guevill	e Rd	1615 - 1630		3		0	(	)	3	1615 - 1715	1	1	:	2	(	0	13
Day/Dat	te	: Tues	day 1	st May	y 2018			1630 - 1645		3		0	(	)	3	1630 - 1730	1	0	:	2	-	0	12
								1645 - 1700		3		0	(	)	3	1645 - 1745	1	0	;	3	Ī	0	13
								1700 - 1715		2		2	(	)	4	1700 - 1800	7	7	;	3	(	0	10
								1715 - 1730		2		0	(	)	2	1715 - 1815	8	8	;	3	-	0	11
								1730 - 1745		3		1		)	4	1730 - 1830	1	0		4		0	14
								1745 - 1800		0		0		)	0								
								1800 - 1815		3		2		)	5	PEAK HR	1	0	;	3	(	)	13
								1815 - 1830		4		1	1	)	5								
								Per End	3	9		8		)	47								
Lights	WE	ST	NO	RTH	E/	ST		Heavies	w	EST	NO	RTH	ΕΛ	ST		Combined	WE	EST	NO	RTH	ΕΛ	ST	1
Ligitio				ueville		ueville		<u>ricavics</u>			1	ueville	Longu			<u>oombiica</u>				ıeville		ieville	1
	Rive	er Rd	•	₹d	_	Rd			Rive	er Rd	_	₹d	R				Rive	r Rd	_	?d	_	?d	
Time Per	<u>T</u>	L	<u>R</u>	<u>L</u>	<u>R</u>	<u>T</u>	TOT	Time Per	<u>T</u>	L	R	L	R	T	тот	Time Per	<u>T</u>	L	R	<u>L</u>	<u>R</u>	<u>T</u>	TOT
1530 - 1545	128	19	0	69	65	258	539	1530 - 1545	0	0	3	1	0	2	6	1530 - 1545	128	19	3	70	65	260	545
1545 - 1600	121	18	1	56	53	275	524	1545 - 1600	2	3	5	2	2	3	17	1545 - 1600	123	21	6	58	55	278	541
1600 - 1615	103	16	0	60	59	287	525	1600 - 1615	4	0	3	0	2	1	10	1600 - 1615	107	16	3	60	61	288	535
1615 - 1630	112	16	0	56	56	294	534	1615 - 1630	2	1	3	2	3	1	12	1615 - 1630	114	17	3	58	59	295	546
1630 - 1645	125	13	0	84	67	273	562	1630 - 1645	1	2	0	0	0	2	5	1630 - 1645	126	15	0	84	67	275	567
1645 - 1700	139	23	0	79	76	342	659	1645 - 1700	0	0	1	1	2	1	5	1645 - 1700	139	23	1	80	78	343	664
1700 - 1715	154	22	1	85	74	258	594	1700 - 1715	1	0	1	0	1	0	3	1700 - 1715	155	22	2	85	75	258	597
1715 - 1730	148	21	0	64	65	338	636	1715 - 1730	1	2	1	1	1	0	6	1715 - 1730	149	23	1	65	66	338	642
1730 - 1745	170	15	0	93	70	328	676	1730 - 1745	1	1	0	1	0	0	3	1730 - 1745	171	16	0	94	70	328	679
1745 - 1800	149	23	0	64	64	294	594	1745 - 1800	2	1	4	0	1	0	8	1745 - 1800	151	24	4	64	65	294	602
1800 - 1815	168	15	0	76	65	257	581	1800 - 1815	1	1	0	1	0	0	3	1800 - 1815	169	16	0	77	65	257	584
1815 - 1830	133	14	0	65	60	215	487	1815 - 1830	0	0	2	1	1	0	4	1815 - 1830	133	14	2	66	61	215	491
Per End	1650	215	2	851	774	3419	6911	Per End	15	11	23	10	13	10	82	Per End	1665	226	25	861	787	3429	6993
Lights	WE	ST	NO	RTH	E/	ST		<u>Heavies</u>	WI	EST	NO	RTH	EA	ST		Combined	WE	ST	NO	RTH	EA	ST	
	Rive	er Rd	•	ueville Rd	_	ueville Rd			Rive	er Rd	_	ueville Rd	Longu R	ieville d			Rive	r Rd	_	ieville Id	_	ieville Id	
Peak Per	I	L	R	L	<u>R</u>	I	тот	Peak Per	I	L	<u>R</u>	L	R	I	тот	Peak Per	I	L	R	L	<u>R</u>	I	тот
1530 - 1630	464	69	1	241	233	1114	2122	1530 - 1630	8	4	14	5	7	7	45	1530 - 1630	<u>4</u> 72	73	15	246	240	1121	2167
1545 - 1645	461	63	1	256	235	1129	2145	1545 - 1645	9	6	11	4	7	7	44	1545 - 1645	470	69	12	260	242	1136	2189
1600 - 1700	479	68	0	279	258	1196	2280	1600 - 1700	7	3	7	3	7	5	32	1600 - 1700	486	71	7	282	265	1201	2312
1615 - 1715	530	74	1	304	273	1167	2349	1615 - 1715	4	3	5	3	6	4	25	1615 - 1715	534	77	6	307	279	1171	2374
1630 - 1730	566	79	1	312	282	1211	2451	1630 - 1730	3	4	3	2	4	3	19	1630 - 1730	569	83	4	314	286	1214	2470
1645 - 1745	611	81	1	321	285	1266	2565	1645 - 1745	3	3	3	3	4	1	17	1645 - 1745	614	84	4	324	289	1267	2582
1700 - 1800	621	81	1	306	273	1218	2500	1700 - 1800	5	4	6	2	3	0	20	1700 - 1800	626	85	7	308	276	1218	2520
1715 - 1815	635	74	0	297	264	1217	2487	1715 - 1815	5	5	5	3	2	0	20	1715 - 1815	640	79	5	300	266	1217	2507
1730 - 1830	620	67	0	298	259	1094	2338	1730 - 1830	4	3	6	3	2	0	18	1730 - 1830	624	70	6	301	261	1094	2356
																1							





## APPENDIX C

## **RMS IDM DATA**



# **Report: Periodic statistics for site 936**

#### 15 minute intervals

From 12:00:00 AM to 11:59:59 PM, on 15 August 2018

Period: 12:00:00 AM to 12:15:00 AM

Data	Freq.	Min	Max	Avg	Total
? phase	1	5	5	5	5
A phase	4	17	669	215	860
C phase	1	12	12	12	12
D phase	2	11	12	11	23

Period: 12:15:00 AM to 12:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	2	253	635	444	888
C phase	1	12	12	12	12

Period: 12:30:00 AM to 12:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 12:45:00 AM to 1:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	2	151	738	444	889
D phase	1	11	11	11	11

Period: 1:00:00 AM to 1:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 1:15:00 AM to 1:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 1:30:00 AM to 1:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 1:45:00 AM to 2:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 2:00:00 AM to 2:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 2:15:00 AM to 2:30:00 AM

1 Clida. 2.10.00 Aivi to 2.50.00 Aivi							
Data	Freq.	Min	Max	Avg	Total		
A phase	1	900	900	900	900		

Period: 2:30:00 AM to 2:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 2:45:00 AM to 3:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 3:00:00 AM to 3:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 3:15:00 AM to 3:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 3:30:00 AM to 3:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	2	107	781	444	888
C phase	1	12	12	12	12

Period: 3:45:00 AM to 4:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	2	197	691	444	888
C phase	1	12	12	12	12

Period: 4:00:00 AM to 4:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	2	239	649	444	888
D phase	1	12	12	12	12

Period: 4:15:00 AM to 4:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	3	95	660	292	876
C phase	2	12	12	12	24

Period: 4:30:00 AM to 4:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 4:45:00 AM to 5:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	3	73	507	292	876
C phase	2	12	12	12	24

Period: 5:00:00 AM to 5:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	4	20	699	216	864
C phase	3	12	12	12	36

Period: 5:15:00 AM to 5:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	8	16	211	102	816
C phase	6	12	12	12	72
D phase	1	12	12	12	12

Period: 5:30:00 AM to 5:45:00 AM

Data	Freq.	Min	Max	Avg	Total		
A phase	6	1	268	140	840		
C phase	4	12	12	12	48		
D phase	1	12	12	12	12		

Period: 5:45:00 AM to 6:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	8	16	206	100	804
C phase	7	7	18	12	85
D phase	1	11	11	11	11
Nominal CL	2	28	60	44	
Active CL	2	28	60	44	
Ped 2	1				

Period: 6:00:00 AM to 6:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	10	44	128	77	775
C phase	9	5	18	12	113
D phase	1	12	12	12	12
Nominal CL	3	28	60	49	
Active CL	3	28	60	49	
Ped 1	1				
Ped 2	2				

Period: 6:15:00 AM to 6:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	10	35	117	70	705
C phase	9	12	18	14	131
D phase	5	12	13	12	64
Ped 2	2				

Period: 6:30:00 AM to 6:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	14	33	102	49	686
C phase	12	3	19	13	162
D phase	4	12	15	13	52
Ped 2	2				

Period: 6:45:00 AM to 7:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	14	24	69	47	670
C phase	11	9	19	14	161
D phase	5	12	18	13	69
Nominal CL	6	69	82	75	
Active CL	6	69	82	75	
Ped 2	1				

Period: 7:00:00 AM to 7:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	12	4	83	52	627
C phase	10	13	22	16	169
D phase	7	13	24	14	104
Nominal CL	10	75	90	84	
Active CL	10	75	90	84	
Ped 2	2				

Period: 7:15:00 AM to 7:30:00 AM

0.104.7.1.0.007.1.1.10.100.7.1.1.							
Data	Freq.	Min	Max	Avg	Total		
A phase	10	51	87	65	652		
C phase	9	16	31	19	175		
D phase	5	13	16	14	73		
Nominal CL	8	90	100	95			
Active CL	8	90	100	95			
Ped 2	4						

Period: 7:30:00 AM to 7:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	10	26	91	62	624
C phase	9	18	28	20	187
D phase	6	12	16	14	89
Nominal CL	7	96	115	105	
Active CL	7	96	115	105	
Ped 1	1				
Ped 2	3				

Period: 7:45:00 AM to 8:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	10	42	88	65	654
C phase	9	18	27	21	191
D phase	4	13	15	13	55
Nominal CL	9	92	105	98	
Active CL	9	92	105	98	
Ped 1	1				
Ped 2	5				

Period: 8:00:00 AM to 8:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	9	36	90	71	647
C phase	9	13	33	20	181
D phase	5	12	16	14	72
Nominal CL	8	100	112	106	
Active CL	8	100	112	106	
Ped 1	1				
Ped 2	7				

Period: 8:15:00 AM to 8:30:00 AM

1 e110d. 0.10.00 AW 10 0.30.00 AW								
Data	Freq.	Min	Max	Avg	Total			
A phase	8	60	100	79	637			
C phase	9	6	19	15	139			
D phase	8	12	23	15	124			
Nominal CL	8	105	120	112				
Active CL	8	105	120	112				

Ped 1	2		
Ped 2	3		

Period: 8:30:00 AM to 8:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	8	43	96	80	642
C phase	8	5	19	17	138
D phase	8	12	16	15	120
Nominal CL	4	117	120	118	
Active CL	4	117	120	118	
Ped 2	4				

Period: 8:45:00 AM to 9:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	9	38	94	71	640
C phase	8	2	19	16	135
D phase	8	14	16	15	125
Nominal CL	7	93	120	107	
Active CL	7	93	120	107	
Ped 1	1				
Ped 2	5				

Period: 9:00:00 AM to 9:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	10	36	142	64	648
C phase	9	12	23	17	154
D phase	7	12	16	14	98
Nominal CL	11	62	97	81	
Active CL	11	62	97	81	
Ped 2	3				

Period: 9:15:00 AM to 9:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	13	14	66	46	606
C phase	12	12	23	17	205
D phase	7	11	13	12	89
Nominal CL	12	64	79	73	
Active CL	12	64	79	73	
Ped 1	2				
Ped 2	4				

Period: 9:30:00 AM to 9:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	14	34	79	45	639
C phase	11	5	24	15	168
D phase	7	12	14	13	93
Nominal CL	6	60	83	73	
Active CL	6	60	83	73	
Ped 1	1				
Ped 2	3				

Period: 9:45:00 AM to 10:00:00 AM

1 Chod: 5:45.55 / tivi to 10:55.55 / tivi									
Data	Freq.	Min	Max	Avg	Total				
A phase	15	11	95	41	625				

C phase	13	12	19	14	185
D phase	7	12	13	12	90
Ped 1	2				
Ped 2	1				

Period: 10:00:00 AM to 10:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	16	21	53	38	612
C phase	9	12	20	17	155
D phase	9	11	20	14	133
Ped 2	4				

Period: 10:15:00 AM to 10:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	15	1	58	41	615
C phase	10	12	20	15	156
D phase	10	11	14	12	129
Nominal CL	7	64	76	71	
Active CL	7	64	76	71	
Ped 2	2				

Period: 10:30:00 AM to 10:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	14	15	95	44	623
C phase	11	12	19	14	159
D phase	9	12	15	13	118
Nominal CL	1	60	60	60	
Active CL	1	60	60	60	
Ped 2	3				

Period: 10:45:00 AM to 11:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	15	28	59	42	638
C phase	12	8	20	15	189
D phase	5	12	18	14	73
Nominal CL	4	60	67	64	
Active CL	4	60	67	64	
Ped 1	2				
Ped 2	3				

Period: 11:00:00 AM to 11:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	14	29	96	41	583
C phase	12	4	20	14	171
D phase	11	11	19	13	146
Ped 1	2				
Ped 2	3				

Period: 11:15:00 AM to 11:30:00 AM

Period. 11.15.00 Aivi to 11.50.00 Aivi								
Data	Freq.	Min	Max	Avg	Total			
A phase	15	28	54	41	629			
C phase	10	8	20	14	146			
D phase	9	12	17	13	125			
Nominal CL	1	69	69	69				

Active CL	1	69	69	69	
Ped 2	2				

Period: 11:30:00 AM to 11:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	15	10	57	40	610
C phase	13	2	20	13	173
D phase	9	11	17	13	117
Nominal CL	7	60	67	63	
Active CL	7	60	67	63	

Period: 11:45:00 AM to 12:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	18	46	37	602
C phase	13	12	20	14	194
D phase	8	12	15	13	104
Ped 1	1				
Ped 2	2				

Period: 12:00:00 PM to 12:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	15	15	105	41	623
C phase	10	12	20	16	165
D phase	8	12	20	14	112
Ped 2	2				

Period: 12:15:00 PM to 12:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	14	20	104	43	613
C phase	13	12	21	16	210
D phase	6	12	13	12	77
Nominal CL	7	60	75	67	
Active CL	7	60	75	67	
Ped 2	1				

Period: 12:30:00 PM to 12:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	13	51	38	613
C phase	13	12	20	13	181
D phase	8	12	16	13	106
Ped 1	1				

Period: 12:45:00 PM to 1:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	15	18	101	43	645
C phase	10	12	20	14	144
D phase	9	11	14	12	111
Ped 1	1				
Ped 2	1				

Period: 1:00:00 PM to 1:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	15	21	114	43	649

C phase	8	12	20	13	109
D phase	11	11	16	12	142

Period: 1:15:00 PM to 1:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	15	22	113	42	631
C phase	12	12	18	13	163
D phase	8	11	18	13	106
Ped 2	1				

Period: 1:30:00 PM to 1:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	15	22	97	42	636
C phase	12	12	19	13	159
D phase	8	12	15	13	105
Ped 1	1				
Ped 2	1				

Period: 1:45:00 PM to 2:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	21	48	37	604
C phase	14	12	20	13	195
D phase	8	12	13	12	101
Ped 2	1				

Period: 2:00:00 PM to 2:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	15	14	101	40	613
C phase	13	12	20	14	182
D phase	8	12	16	13	105
Ped 1	1				

Period: 2:15:00 PM to 2:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	14	21	64	43	609
C phase	12	12	18	14	172
D phase	10	3	14	11	119
Nominal CL	7	60	80	71	
Active CL	7	60	80	71	
Ped 1	2				
Ped 2	3				

Period: 2:30:00 PM to 2:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	14	34	59	43	614
C phase	12	3	20	14	170
D phase	9	10	14	12	116
Nominal CL	5	67	82	74	
Active CL	5	67	82	74	
Ped 2	1				

Period: 2:45:00 PM to 3:00:00 PM

0.104. 2.10.00 1 111 to 0.00.00 1 111								
Data Freq.	Min	Max	Avg	Total				

A phase	14	29	65	39	550
C phase	14	8	21	13	188
D phase	12	12	20	13	162
Nominal CL	6	60	74	67	
Active CL	6	60	74	67	
Ped 2	2				

Period: 3:00:00 PM to 3:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	13	29	65	42	558
C phase	13	5	20	13	179
D phase	12	12	16	13	163
Nominal CL	7	60	94	77	
Active CL	7	60	94	77	
Ped 1	1				
Ped 2	3				

Period: 3:15:00 PM to 3:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	11	39	70	56	619
C phase	10	4	19	14	142
D phase	10	13	16	13	139
Nominal CL	8	74	92	82	
Active CL	8	74	92	82	
Ped 2	3				

Period: 3:30:00 PM to 3:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	12	19	75	52	633
C phase	10	4	18	13	138
D phase	9	13	19	14	129
Nominal CL	11	62	99	77	
Active CL	11	62	99	77	
Ped 2	2				

Period: 3:45:00 PM to 4:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	13	3	89	43	559
C phase	11	12	19	17	191
D phase	10	13	17	15	150
Nominal CL	5	60	105	93	
Active CL	5	60	105	93	
Ped 1	2				
Ped 2	7				

Period: 4:00:00 PM to 4:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	10	52	81	64	641
C phase	8	17	19	18	144
D phase	8	12	17	14	115
Nominal CL	9	90	100	94	
Active CL	9	90	100	94	
Ped 1	2				
Ped 2	4				

Period: 4:15:00 PM to 4:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	11	4	85	55	606
C phase	10	15	20	17	175
D phase	8	13	18	14	119
Nominal CL	10	86	103	94	
Active CL	10	86	103	94	
Ped 2	4				

Period: 4:30:00 PM to 4:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	12	16	64	49	599
C phase	11	12	21	15	175
D phase	9	13	15	14	126
Nominal CL	9	71	96	82	
Active CL	9	71	96	82	
Ped 1	2				
Ped 2	2				

Period: 4:45:00 PM to 5:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	12	34	75	50	608
C phase	11	12	19	14	162
D phase	10	12	14	13	130
Nominal CL	12	63	86	75	
Active CL	12	63	86	75	
Ped 1	1				
Ped 2	3				

Period: 5:00:00 PM to 5:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	9	58	88	69	621
C phase	8	15	19	17	139
D phase	10	1	20	14	140
Nominal CL	7	92	111	101	
Active CL	7	92	111	101	
Ped 1	3				
Ped 2	3				

Period: 5:15:00 PM to 5:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	10	20	97	61	611
C phase	9	15	19	17	156
D phase	9	5	19	14	133
Nominal CL	9	89	106	97	
Active CL	9	89	106	97	
Ped 1	1				
Ped 2	4				

Period: 5:30:00 PM to 5:45:00 PM

1 0110d: 0:00:00 1 W to 0:10:00 1 W							
Data	Freq.	Min	Max	Avg	Total		
A phase	8	47	93	80	645		
C phase	8	17	22	19	152		
D phase	7	6	17	14	103		
Nominal CL	5	112	120	116			

Active CL	5	112	120	116	
Ped 1	4				
Ped 2	4				·

Period: 5:45:00 PM to 6:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	8	56	88	76	613
C phase	8	17	19	18	147
D phase	9	10	18	15	140
Nominal CL	8	98	120	111	
Active CL	8	98	120	111	
Ped 1	2				
Ped 2	5				

Period: 6:00:00 PM to 6:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	12	14	64	53	639
C phase	9	12	19	14	133
D phase	10	1	16	12	128
Nominal CL	10	60	97	78	
Active CL	10	60	97	78	
Ped 2	2				

Period: 6:15:00 PM to 6:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	13	15	62	42	558
C phase	12	12	19	15	186
D phase	12	12	14	13	156
Nominal CL	9	60	88	76	
Active CL	9	60	88	76	
Ped 1	2				
Ped 2	4				

Period: 6:30:00 PM to 6:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	1	55	35	575
C phase	12	12	20	14	170
D phase	11	12	21	14	155
Nominal CL	3	70	77	74	
Active CL	3	70	77	74	
Ped 1	1				
Ped 2	2				

Period: 6:45:00 PM to 7:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	14	22	58	42	588
C phase	11	12	19	15	168
D phase	11	12	15	13	144
Nominal CL	9	60	78	68	
Active CL	9	60	78	68	
Ped 2	4				

Period: 7:00:00 PM to 7:15:00 PM

1 Clidd. 7.00:00 1 W to 7.10:00 1 W								
Data	Freq.	Min	Max	Avg	Total			

A phase	14	3	97	45	640
C phase	9	12	18	14	134
D phase	9	13	17	14	126
Nominal CL	8	60	75	68	
Active CL	8	60	75	68	
Ped 2	3				

Period: 7:15:00 PM to 7:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	15	15	105	44	671
C phase	10	12	17	13	132
D phase	8	11	13	12	97
Nominal CL	1	60	60	60	
Active CL	1	60	60	60	

Period: 7:30:00 PM to 7:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	15	26	94	44	661
C phase	8	12	18	13	111
D phase	10	11	15	12	128
Nominal CL	1	28	28	28	
Active CL	1	28	28	28	
Ped 2	1				

Period: 7:45:00 PM to 8:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	14	31	101	45	640
C phase	11	12	19	13	146
D phase	9	10	20	12	114
Nominal CL	1	60	60	60	
Active CL	1	60	60	60	
Ped 2	1				

Period: 8:00:00 PM to 8:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	10	36	169	71	711
C phase	10	12	19	13	139
D phase	5	3	12	10	50
Ped 2	3				

Period: 8:15:00 PM to 8:30:00 PM

1 CHOQ: 0.10.00 1 W to 0.00.00 1 W							
Data	Freq.	Min	Max	Avg	Total		
A phase	13	26	152	57	746		
C phase	8	12	17	13	107		
D phase	5	1	12	9	47		
Nominal CL	2	28	60	44			
Active CL	2	28	60	44			

Period: 8:30:00 PM to 8:45:00 PM

Feliod. 6.30.00 FM to 6.43.00 FM								
Data	Freq.	Min	Max	Avg	Total			
A phase	14	22	102	52	729			
C phase	10	12	18	13	136			
D phase	3	11	13	11	35			
Nominal CL	2	28	60	44				

Active CL	2	28	60	44	
Ped 2	1				

Period: 8:45:00 PM to 9:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	9	33	110	86	781
C phase	4	12	16	14	56
D phase	5	11	14	12	63

Period: 9:00:00 PM to 9:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	10	42	162	78	781
C phase	7	12	18	13	97
D phase	2	11	11	11	22
Ped 2	1				

Period: 9:15:00 PM to 9:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	7	46	189	115	808
C phase	5	12	20	13	69
D phase	2	11	12	11	23
Nominal CL	2	28	60	44	
Active CL	2	28	60	44	

Period: 9:30:00 PM to 9:45:00 PM

0.104.0.00.00.1							
Data	Freq.	Min	Max	Avg	Total		
A phase	9	42	341	88	794		
C phase	4	6	17	11	47		
D phase	5	11	13	11	59		
Nominal CL	2	28	60	44			
Active CL	2	28	60	44			

Period: 9:45:00 PM to 10:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	10	16	168	75	755
C phase	9	6	18	13	120
D phase	2	12	13	12	25
Nominal CL	2	28	60	44	
Active CL	2	28	60	44	
Ped 2	1				

Period: 10:00:00 PM to 10:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	9	42	165	86	775
C phase	6	7	18	14	87
D phase	3	11	14	12	38
Ped 2	1				

Period: 10:15:00 PM to 10:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	7	11	225	116	814
C phase	6	6	16	12	73
D phase	1	13	13	13	13

Nominal CL	2	28	60	44	
Active CL	2	28	60	44	

Period: 10:30:00 PM to 10:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	7	16	298	117	822
C phase	6	12	18	13	78
Nominal CL	1	28	28	28	
Active CL	1	28	28	28	
Ped 2	1				

Period: 10:45:00 PM to 11:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	4	35	716	216	865
D phase	3	11	12	11	35

Period: 11:00:00 PM to 11:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	8	30	213	102	819
C phase	4	12	12	12	48
D phase	3	11	11	11	33
Nominal CL	2	28	60	44	
Active CL	2	28	60	44	

Period: 11:15:00 PM to 11:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	6	6	240	139	834
C phase	4	12	18	13	54
D phase	1	12	12	12	12
Ped 2	1				

Period: 11:30:00 PM to 11:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 11:45:00 PM to Unknown

Data	Freq.	Min	Max	Avg	Total
? phase	1	5	5	5	5
A phase	2	118	764	441	882
C phase	1	12	12	12	12

# TCS 708

LONGUEVILLE

215E

WILL

SS=26

## 3 PHASES





# **Report: Periodic statistics for site 708**

## 15 minute intervals

From 12:00:00 AM to 11:59:59 PM, on 15 August 2018

Period: 12:00:00 AM to 12:15:00 AM

Data	Freq.	Min	Max	Avg	Total
? phase	1	8	8	8	8
A phase	7	11	296	116	818
B phase	6	11	14	12	74

Period: 12:15:00 AM to 12:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	6	44	295	140	842
B phase	5	11	13	11	58

Period: 12:30:00 AM to 12:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	2	319	569	444	888
B phase	1	12	12	12	12

Period: 12:45:00 AM to 1:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	2	63	822	442	885
B phase	1	15	15	15	15

Period: 1:00:00 AM to 1:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	2	319	569	444	888
B phase	1	12	12	12	12

Period: 1:15:00 AM to 1:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	2	240	648	444	888
B phase	1	12	12	12	12

Period: 1:30:00 AM to 1:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	3	191	464	292	876
B phase	2	12	12	12	24

Period: 1:45:00 AM to 2:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 2:00:00 AM to 2:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 2:15:00 AM to 2:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	2	345	543	444	888
B phase	1	12	12	12	12

Period: 2:30:00 AM to 2:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 2:45:00 AM to 3:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 3:00:00 AM to 3:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	1	900	900	900	900

Period: 3:15:00 AM to 3:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	2	416	473	444	889
B phase	1	11	11	11	11

Period: 3:30:00 AM to 3:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	3	12	587	290	870
B phase	1	11	11	11	11
C phase	1	19	19	19	19
IP3	1				
Ped 2	1				

Period: 3:45:00 AM to 4:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	2	268	618	443	886
B phase	1	14	14	14	14
IP2	1				
IP3	1				

Period: 4:00:00 AM to 4:15:00 AM

1 0110d: 1:00:00 / W/ to 1:10:00 / W/								
Data	Freq.	Min	Max	Avg	Total			
A phase	3	39	624	292	876			
B phase	2	12	12	12	24			
IP2	2							
IP3	2							

Period: 4:15:00 AM to 4:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	3	180	512	291	875
B phase	2	12	13	12	25
IP2	3				
IP3	4				

Period: 4:30:00 AM to 4:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	6	13	303	140	841
B phase	5	11	13	11	59
IP2	2				
IP3	3				

Period: 4:45:00 AM to 5:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	6	20	291	139	837
B phase	5	12	14	12	63
IP3	1				

Period: 5:00:00 AM to 5:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	3	111	613	292	878
B phase	2	11	11	11	22
IP3	1				

Period: 5:15:00 AM to 5:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	10	16	239	78	786
B phase	9	11	14	12	114
IP2	1				
IP3	1				

Period: 5:30:00 AM to 5:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	11	30	128	70	773
B phase	9	11	13	12	109
C phase	1	18	18	18	18
IP2	2				
IP3	1				
Ped 2	1				

Period: 5:45:00 AM to 6:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	13	24	157	55	724
B phase	12	11	19	13	157
C phase	1	19	19	19	19
Nominal CL	2	28	60	44	
Active CL	2	28	60	44	
IP2	4				
IP3	3				
Ped 2	1				

Data	Freq.	Min	Max	Avg	Total
A phase	14	4	224	49	699
B phase	13	11	16	12	162
C phase	2	19	20	19	39
Nominal CL	3	28	60	49	
Active CL	3	28	60	49	
IP2	2				
IP3	2				

IP4	1		
Ped 2	2		

Period: 6:15:00 AM to 6:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	14	27	101	46	653
B phase	13	11	17	14	187
C phase	3	20	20	20	60
IP4	1				
IP5	1				
IP6	1				
IP7	2				
IP8	1				
Ped 2	3				

Period: 6:30:00 AM to 6:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	15	29	59	42	635
B phase	15	11	20	15	226
C phase	2	19	20	19	39
IP7	1				
IP8	1				
Ped 2	2				

Period: 6:45:00 AM to 7:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	14	12	63	46	647
B phase	13	12	21	14	193
C phase	3	20	20	20	60
Nominal CL	6	69	82	75	
Active CL	6	69	82	75	
IP7	2				
IP8	2				
Ped 2	3				

Period: 7:00:00 AM to 7:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	12	15	79	51	620
B phase	11	12	32	18	200
C phase	4	20	20	20	80
Nominal CL	10	75	90	84	
Active CL	10	75	90	84	
IP7	1				
IP8	1				
Ped 2	4				

Period: 7:15:00 AM to 7:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	10	50	83	68	681
B phase	9	13	31	22	199
C phase	1	20	20	20	20
Nominal CL	8	90	100	95	
Active CL	8	90	100	95	
IP8	1				
Ped 2	1				

Period: 7:30:00 AM to 7:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	10	22	98	62	622
B phase	9	15	26	19	178
C phase	5	20	20	20	100
Nominal CL	7	96	115	105	
Active CL	7	96	115	105	
IP6	2				
IP7	3				
IP8	2				
Ped 2	5				

Period: 7:45:00 AM to 8:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	10	39	91	65	659
B phase	9	15	24	20	181
C phase	3	20	20	20	60
Nominal CL	9	92	105	98	
Active CL	9	92	105	98	
IP5	1				
IP6	4				
IP7	2				
Ped 1	1				
Ped 2	3				

Period: 8:00:00 AM to 8:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	9	25	90	66	598
B phase	9	18	37	22	202
C phase	5	20	20	20	100
Nominal CL	8	100	112	106	
Active CL	8	100	112	106	
IP5	1				
IP6	2				
IP7	2				
IP8	1				
Ped 1	3				
Ped 2	5				

Period: 8:15:00 AM to 8:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	8	56	96	74	592
B phase	9	1	28	21	197
C phase	6	11	20	18	111
Nominal CL	8	105	120	112	
Active CL	8	105	120	112	
IP5	2				
IP6	2				
IP7	1				
Ped 1	2				
Ped 2	6				

Period: 8:30:00 AM to 8:45:00 AM

Data	Freq.	Min	Max	Avg	Total			

A phase	8	62	95	81	654
B phase	7	16	31	25	177
C phase	4	9	20	17	69
Nominal CL	4	117	120	118	
Active CL	4	117	120	118	
IP5	2				
IP6	2				
IP7	1				
Ped 1	2				
Ped 2	3				

Period: 8:45:00 AM to 9:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	9	21	100	69	628
B phase	9	3	26	19	172
C phase	5	20	20	20	100
Nominal CL	7	93	120	107	
Active CL	7	93	120	107	
IP6	2				
IP7	4				
IP8	1				
Ped 1	1				
Ped 2	5				

Period: 9:00:00 AM to 9:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	11	38	79	55	610
B phase	11	16	29	20	230
C phase	3	20	20	20	60
Nominal CL	11	62	97	81	
Active CL	11	62	97	81	
IP6	2				
IP7	4				
IP8	1				
Ped 1	1				
Ped 2	3				

Period: 9:15:00 AM to 9:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	13	6	65	43	570
B phase	13	12	28	17	230
C phase	5	20	20	20	100
Nominal CL	12	64	79	73	
Active CL	12	64	79	73	
IP6	1				
IP7	4				
IP8	2				
Ped 2	5				

Period: 9:30:00 AM to 9:45:00 AM

1 e110d. 9.30.00 AW to 9.40.00 AW							
Data	Freq.	Min	Max	Avg	Total		
A phase	14	25	71	43	610		
B phase	14	12	30	17	251		
C phase	2	19	20	19	39		
Nominal CL	6	60	83	73			
Active CL	6	60	83	73			

IP5	1		
IP6	2		
IP7	3		
IP8	1		
Ped 2	2		

Period: 9:45:00 AM to 10:00:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	15	22	47	37	563
B phase	16	1	25	16	257
C phase	4	20	20	20	80
IP5	2				
IP6	3				
IP7	1				
Ped 2	4				

Period: 10:00:00 AM to 10:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	15	25	43	36	552
B phase	15	13	31	19	288
C phase	3	20	20	20	60
IP5	1				
IP6	4				
IP7	2				
Ped 2	3				

Period: 10:15:00 AM to 10:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	14	21	58	40	571
B phase	14	13	23	17	249
C phase	4	20	20	20	80
Nominal CL	8	60	76	70	
Active CL	8	60	76	70	
IP4	1				
IP5	4				
IP6	4				
IP7	1				
Ped 2	4				

Period: 10:30:00 AM to 10:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	16	8	60	35	568
B phase	15	12	26	16	253
C phase	4	19	20	19	79
IP5	2				
IP6	5				
IP7	4				
Ped 2	4				

Period: 10:45:00 AM to 11:00:00 AM

Data	Freq.	Min	Max	Ava	Total
A phase	16	4	56	34	550
B phase	15	11	29	19	290
C phase	3	20	20	20	60
Nominal CL	4	60	67	64	

Active CL	4	60	67	64	
IP4	1				
IP5	2				
IP6	1				
IP7	1				
Ped 1 Ped 2	1				
Ped 2	3				

Period: 11:00:00 AM to 11:15:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	16	3	49	36	585
B phase	15	11	24	17	255
C phase	3	20	20	20	60
IP5	3				
IP6	5				
IP7	2				
Ped 2	3				

Period: 11:15:00 AM to 11:30:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	16	6	46	36	589
B phase	15	14	34	19	291
C phase	1	20	20	20	20
Nominal CL	1	69	69	69	
Active CL	1	69	69	69	
IP5	4				
IP6	5				
IP7	1				
Ped 2	1				

Period: 11:30:00 AM to 11:45:00 AM

Data	Freq.	Min	Max	Avg	Total
A phase	15	29	55	42	633
B phase	14	13	27	19	267
Nominal CL	7	60	67	63	
Active CL	7	60	67	63	
IP4	1				
IP5	3				
IP6	3				
IP7	2				

Period: 11:45:00 AM to 12:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	11	48	34	550
B phase	15	12	39	20	310
C phase	2	20	20	20	40
IP3	1				
IP4	2				
IP5	4				
IP6	2				
Ped 1	1				
Ped 2	2				

Period: 12:00:00 PM to 12:15:00 PM

Data	Freq.	Min	Max	Avg	Total				

A phase	16	11	42	34	547
B phase	15	14	35	20	313
C phase	2	20	20	20	40
IP5	4				
IP6	4				
Ped 2	2				

Period: 12:15:00 PM to 12:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	15	13	58	39	589
B phase	14	12	26	17	251
C phase	3	20	20	20	60
Nominal CL	7	60	75	67	
Active CL	7	60	75	67	
IP4	1				
IP5	4				
IP6	4				
IP7	1				
Ped 1	1				
Ped 2	3				

Period: 12:30:00 PM to 12:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	6	49	34	556
B phase	15	12	28	19	285
C phase	3	19	20	19	59
IP5	3				
IP6	4				
IP7	2				
Ped 1	1				
Ped 2	3				

Period: 12:45:00 PM to 1:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	10	45	36	579
B phase	15	13	29	18	281
C phase	2	20	20	20	40
IP4	2				
IP5	5				
IP6	3				
IP7	1				
Ped 1	1				
Ped 2	2				

Period: 1:00:00 PM to 1:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	12	54	36	588
B phase	15	15	24	18	272
C phase	2	20	20	20	40
IP4	3				
IP5	5				
IP6	3				
IP7	1				
Ped 2	2				

Period: 1:15:00 PM to 1:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	8	46	35	562
B phase	15	12	35	18	279
C phase	3	19	20	19	59
IP5	3				
IP6	6				
IP7	2				
Ped 1	1				
Ped 2	3				

Period: 1:30:00 PM to 1:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	15	30	70	41	625
B phase	14	11	23	18	255
C phase	1	20	20	20	20
IP5	1				
IP6	3				
IP7	2				
Ped 1	1				
Ped 2	1				

Period: 1:45:00 PM to 2:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	8	46	34	559
B phase	15	14	25	18	281
C phase	3	20	20	20	60
IP3	2				
IP4	3				
IP5	4				
IP6	2				
Ped 2	3				

Period: 2:00:00 PM to 2:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	9	45	34	550
B phase	15	15	39	18	271
C phase	4	19	20	19	79
IP4	2				
IP5	2				
Ped 2	4				

Period: 2:15:00 PM to 2:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	15	6	62	38	584
B phase	14	14	31	21	296
C phase	1	20	20	20	20
Nominal CL	7	60	80	71	
Active CL	7	60	80	71	
IP4	3				
IP5	3				
Ped 2	1				

Period: 2:30:00 PM to 2:45:00 PM

_	_			_	
Data	Freq.	Min	Max	Avg	Total

A phase	14	32	64	43	608
B phase	14	13	27	19	272
C phase	1	20	20	20	20
Nominal CL	5	67	82	74	
Active CL	5	67	82	74	
IP4	1				
IP5	3				
IP6	2				
IP7	1				
Ped 1	1				
Ped 2	1				

Period: 2:45:00 PM to 3:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	14	24	58	41	581
B phase	14	17	31	21	299
C phase	1	20	20	20	20
Nominal CL	6	60	74	67	
Active CL	6	60	74	67	
IP4	3				
IP5	7				
IP6	3				
Ped 2	1				

Period: 3:00:00 PM to 3:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	13	16	67	39	517
B phase	14	1	42	20	287
C phase	5	16	20	19	96
Nominal CL	7	60	94	77	
Active CL	7	60	94	77	
IP2	1				
IP3	4				
IP4	4				
IP5	2				
Ped 1	2				
Ped 2	5				

Period: 3:15:00 PM to 3:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	12	7	68	46	562
B phase	11	16	33	24	274
C phase	4	4	20	16	64
Nominal CL	8	74	92	82	
Active CL	8	74	92	82	
IP1	1				
IP2	1				
IP3	2				
IP4	1				
Ped 2	3				

Period: 3:30:00 PM to 3:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	12	23	80	49	594
B phase	12	1	34	23	286
C phase	1	20	20	20	20

Nominal CL	11	62	99	77	
Active CL	11	62	99	77	
IP1	2				
IP2	3				
IP3	3				
IP4	1				
Ped 2	1				

Period: 3:45:00 PM to 4:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	11	17	64	42	462
B phase	11	20	64	32	358
C phase	4	20	20	20	80
Nominal CL	6	60	105	94	
Active CL	6	60	105	94	
IP1	1				
IP2	1				
IP3	2				
IP4	1				
Ped 2	4				

Period: 4:00:00 PM to 4:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	10	27	76	57	572
B phase	10	18	61	26	268
C phase	3	20	20	20	60
Nominal CL	8	90	100	93	
Active CL	8	90	100	93	
IP1	1				
IP2	1				
Ped 1	1				
Ped 2	3				

Period: 4:15:00 PM to 4:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	10	34	82	55	559
B phase	10	15	40	26	261
C phase	4	20	20	20	80
Nominal CL	10	86	103	94	
Active CL	10	86	103	94	
IP1	1				
IP2	1				
Ped 2	4				

Period: 4:30:00 PM to 4:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	12	13	77	47	568
B phase	11	13	45	24	272
C phase	3	20	20	20	60
Nominal CL	9	71	96	82	
Active CL	9	71	96	82	
IP1	1				
IP2	2				
IP3	2				
IP4	1				
Ped 2	3				

Period: 4:45:00 PM to 5:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	13	17	67	38	494
B phase	12	16	45	27	326
C phase	4	20	20	20	80
Nominal CL	12	63	86	75	
Active CL	12	63	86	75	
IP2	1				
IP3	3				
IP4	3				
Ped 2	4				

Period: 5:00:00 PM to 5:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	10	31	74	56	560
B phase	9	17	40	26	240
C phase	5	20	20	20	100
Nominal CL	7	92	111	101	
Active CL	7	92	111	101	
IP1	1				
IP2	1				
IP3	1				
IP4	1				
Ped 1	1				
Ped 2	5				

Period: 5:15:00 PM to 5:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	10	31	74	50	509
B phase	9	14	64	36	331
C phase	3	20	20	20	60
Nominal CL	9	89	106	97	
Active CL	9	89	106	97	
IP1	2				
IP2	1				
Ped 1	2				
Ped 2	3				

Period: 5:30:00 PM to 5:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	8	13	98	54	435
B phase	8	18	82	48	390
C phase	4	15	20	18	75
Nominal CL	5	112	120	116	
Active CL	5	112	120	116	
IP1	1				
Ped 1	1				
Ped 2	4				

Period: 5:45:00 PM to 6:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	9	3	84	52	475
B phase	8	26	67	45	360
C phase	4	5	20	16	65

Nominal CL	8	98	120	111	
Active CL	8	98	120	111	
IP1	1				
Ped 1	1				
Ped 2	3				

Period: 6:00:00 PM to 6:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	12	29	70	51	621
B phase	12	4	39	21	259
C phase	1	20	20	20	20
Nominal CL	10	60	97	78	
Active CL	10	60	97	78	
IP1	3				
IP2	3				
IP3	1				
Ped 2	1				

Period: 6:15:00 PM to 6:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	12	22	72	47	568
B phase	13	8	30	22	292
C phase	2	20	20	20	40
Nominal CL	9	60	88	76	
Active CL	9	60	88	76	
IP2	1				
IP3	4				
IP4	3				
Ped 1	2				
Ped 2	2				

Period: 6:30:00 PM to 6:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	15	2	63	38	580
B phase	15	9	31	18	280
C phase	2	20	20	20	40
Nominal CL	3	70	77	74	
Active CL	3	70	77	74	
IP2	1				
IP3	1				
IP4	1				
IP5	1				
IP6	2				
IP7	1				
Ped 2	2				

Period: 6:45:00 PM to 7:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	14	22	63	44	620
B phase	14	10	26	18	260
C phase	1	20	20	20	20
Nominal CL	9	60	78	68	
Active CL	9	60	78	68	
IP4	1				
IP5	4				
IP6	4				

IP7	1		
Ped 2	1		

Period: 7:00:00 PM to 7:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	14	23	58	44	624
B phase	14	9	21	16	236
C phase	2	20	20	20	40
Nominal CL	8	60	75	68	
Active CL	8	60	75	68	
IP5	3				
IP6	4				
IP7	2				
Ped 2	2				

Period: 7:15:00 PM to 7:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	6	48	38	613
B phase	14	14	24	17	247
C phase	2	20	20	20	40
Nominal CL	1	60	60	60	
Active CL	1	60	60	60	
IP5	3				
IP6	3				
IP7	1				
Ped 2	2				

Period: 7:30:00 PM to 7:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	15	6	104	43	648
B phase	15	8	21	15	232
C phase	1	20	20	20	20
Nominal CL	1	28	28	28	
Active CL	1	28	28	28	
IP4	2				
IP5	4				
IP6	2				
Ped 1	2				
Ped 2	1				

Period: 7:45:00 PM to 8:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	15	24	71	43	655
B phase	15	8	31	16	245
Nominal CL	1	60	60	60	
Active CL	1	60	60	60	
IP2	1				
IP3	4				
IP4	4				

Period: 8:00:00 PM to 8:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	14	67	42	678
B phase	15	12	19	14	222
IP4	1				

IP5	2		
IP6	4		
IP7	2		

Period: 8:15:00 PM to 8:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	25	60	41	662
B phase	15	12	23	15	238
Nominal CL	2	28	60	44	
Active CL	2	28	60	44	
IP1	2				
IP2	2				
IP3	1				
IP4	1				
IP5	1				
IP6	1				

Period: 8:30:00 PM to 8:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	17	8	50	38	654
B phase	16	12	21	15	246
Nominal CL	2	28	60	44	
Active CL	2	28	60	44	
IP1	3				
IP2	4				
IP3	1				

Period: 8:45:00 PM to 9:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	11	3	229	67	738
B phase	10	12	21	16	162
IP1	1				
IP2	4				
IP3	3				
IP4	1				

Period: 9:00:00 PM to 9:15:00 PM

1 enda. 9.00.00 f W to 9.13.00 f W								
Data	Freq.	Min	Max	Avg	Total			
A phase	15	5	97	46	692			
B phase	14	12	25	14	208			
IP1	1							
IP2	3							
IP3	2							
IP4	1							

Period: 9:15:00 PM to 9:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	17	1	72	40	683
B phase	16	11	20	13	217
Nominal CL	2	28	60	44	
Active CL	2	28	60	44	
IP1	2				
IP2	3				
IP3	2				
IP4	1				

Period: 9:30:00 PM to 9:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	16	7	106	43	690
B phase	15	12	21	14	210
Nominal CL	2	28	60	44	
Active CL	2	28	60	44	
IP2	2				
IP3	2				

Period: 9:45:00 PM to 10:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	18	13	50	34	629
B phase	17	11	28	14	251
C phase	1	20	20	20	20
Nominal CL	2	28	60	44	
Active CL	2	28	60	44	
IP1	1				
IP2	3				
IP3	1				
Ped 2	1				

Period: 10:00:00 PM to 10:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	12	31	194	62	746
B phase	11	11	18	14	154
IP2	3				
IP3	4				
IP4	2				
IP5	1				

Period: 10:15:00 PM to 10:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	12	1	158	64	770
B phase	11	11	14	11	130
Nominal CL	2	28	60	44	
Active CL	2	28	60	44	
IP1	1				
IP2	2				
IP3	1				
IP4	1				
IP5	1				

Period: 10:30:00 PM to 10:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	10	11	261	79	791
B phase	9	11	13	12	109
Nominal CL	1	28	28	28	
Active CL	1	28	28	28	
IP1	3				
IP2	5				
IP3	1				

Period: 10:45:00 PM to 11:00:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	10	12	155	78	785
B phase	9	11	17	12	115
IP1	5				
IP2	5				

Period: 11:00:00 PM to 11:15:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	9	6	352	89	801
B phase	8	11	15	12	99
Nominal CL	2	28	60	44	
Active CL	2	28	60	44	
IP1	4				
IP2	3				

Period: 11:15:00 PM to 11:30:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	7	15	374	118	827
B phase	6	11	13	12	73
IP1	3				
IP2	2				

Period: 11:30:00 PM to 11:45:00 PM

Data	Freq.	Min	Max	Avg	Total
A phase	5	4	407	169	848
B phase	4	11	16	13	52
IP1	3				
IP2	3				

Period: 11:45:00 PM to Unknown

Data	Freq.	Min	Max	Avg	Total
? phase	1	11	11	11	11
A phase	7	20	297	116	812
B phase	6	11	17	12	76
IP1	2				
IP2	2				

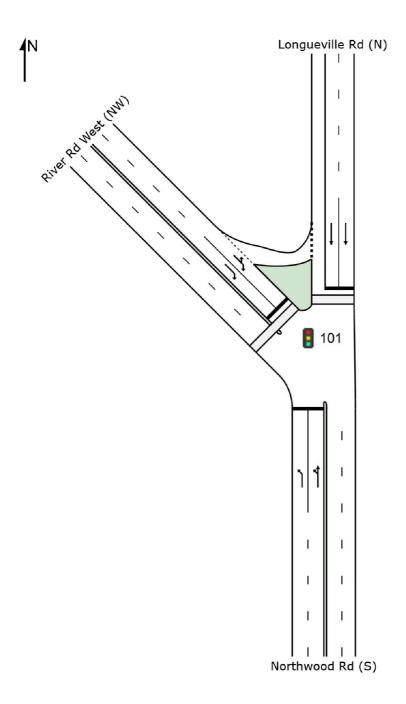
# APPENDIX D

SIDRA MOVEMENT SUMMARIES

# **SITE LAYOUT**

# Site: 101 [LON\_RIVX AM]

Longueville Rd, River Rd & Northwood Rd, Lane Cove Signals - Fixed Time Isolated





Site: 101 [LON\_RIVX AM]

**♦** Network: N101 [Existing AM (Existing Arrangements)]

Longueville Rd, River Rd & Northwood Rd, Lane Cove

Signals - Fixed Time Coordinated Cycle Time = 112 seconds (Network Cycle Time - Program)

Move	Movement Performance - Vehicles														
Mov ID	OD Mov	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed		
Occide	. N. s. stles	veh/h		veh/h	%	v/c	sec		veh	m		per veh	km/h		
South	: North	vood Rd (S	)												
1a	L1	611	8.0	611	8.0	0.367	2.6	LOSA	1.5	10.6	0.05	0.49	45.7		
2	T1	288	1.0	288	1.0	1.030	106.9	LOS F	6.9	49.0	1.00	1.27	1.3		
Appro	ach	899	0.9	899	0.9	1.030	36.0	LOS C	6.9	49.0	0.35	0.74	21.2		
North:	Longu	eville Rd (N	1)												
8	T1	264	8.0	264	8.0	0.957	86.9	LOS F	7.4	52.2	1.00	1.19	1.8		
Appro	ach	264	0.8	264	8.0	0.957	86.9	LOS F	7.4	52.2	1.00	1.19	1.8		
North\	West: R	iver Rd We	est (NW	/)											
27b	L3	179	6.1	179	6.1	0.991	83.8	LOS F	69.4	491.8	1.00	1.23	15.6		
29a	R1	1293	0.1	1293	0.1	0.991	82.7	LOS F	69.4	491.8	1.00	1.23	15.4		
Appro	ach	1472	0.8	1472	8.0	0.991	82.9	LOS F	69.4	491.8	1.00	1.23	15.4		
All Ve	hicles	2635	0.8	2635	8.0	1.030	67.3	LOS E	69.4	491.8	0.78	1.06	15.5		

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 32.5 %

Number of Iterations: 10 (maximum specified: 10)

Move	Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped						
P3	North Full Crossing	6	5.5	LOS A	0.0	0.0	0.31	0.31						
P7	NorthWest Full Crossing	17	50.2	LOS E	0.1	0.1	0.95	0.95						
All Pe	edestrians	23	38.5	LOS D			0.78	0.78						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 101 [LON\_RIVX PM]

 Photomork: N101 [Existing PM] (Existing Arrangements)]

Longueville Rd, River Rd & Northwood Rd, Lane Cove

Move	ment	Performa	nce - \	/ehicle	S								
Mov	OD	Demand	Flows	Arriva	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective A	Average
ID	Mov	Total	HV	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop	Speed
												Rate	
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h
South	: North	wood Rd (S	S)										
1a	L1	1267	0.1	1267	0.1	0.630	7.8	LOSA	6.9	49.0	0.21	0.56	40.9
2	T1	289	1.4	289	1.4	0.630	32.4	LOS C	6.9	49.0	1.00	0.88	4.1
Appro	ach	1556	0.3	1556	0.3	0.630	12.4	LOS A	6.9	49.0	0.36	0.62	35.4
		5.14	• `										
North:	Longu	eville Rd (N	۷)										
8	T1	324	0.9	324	0.9	0.279	17.1	LOS B	5.7	39.9	0.68	0.57	7.8
Appro	ach	324	0.9	324	0.9	0.279	17.1	LOS B	5.7	39.9	0.68	0.57	7.8
North	Noct. B	River Rd We	et (NIV	/\									
			•	,									
27b	L3	84	3.6	84	3.6	0.629	27.0	LOS B	14.6	103.0	0.84	0.84	29.6
29a	R1	614	0.5	614	0.5	0.629	25.3	LOS B	14.6	103.0	0.84	0.83	29.6
Appro	ach	698	0.9	698	0.9	0.629	25.5	LOS B	14.6	103.0	0.84	0.83	29.6
All Ve	hicles	2578	0.5	2578	0.5	0.630	16.5	LOS B	14.6	103.0	0.53	0.67	31.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 19.6 %

Number of Iterations: 10 (maximum specified: 10)

Move	Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped						
P3	North Full Crossing	3	18.7	LOS B	0.0	0.0	0.66	0.66						
P7	NorthWest Full Crossing	10	37.7	LOS D	0.0	0.0	0.93	0.93						
All Pe	edestrians	13	33.3	LOS D			0.87	0.87						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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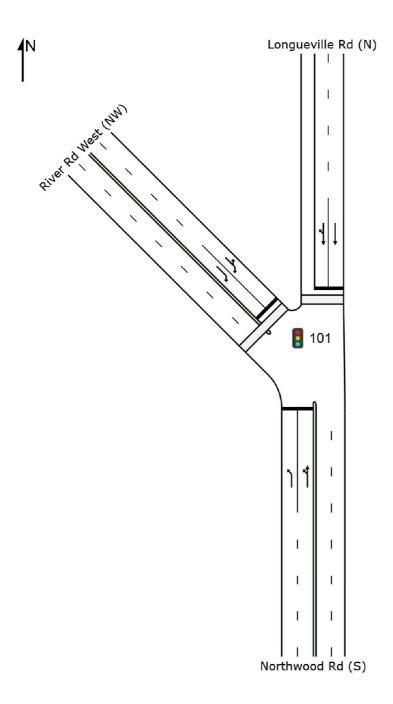
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# **SITE LAYOUT**

# Site: 101 [LON\_RIVP(P) AM]

Longueville Rd, River Rd & Northwood Rd, Lane Cove Signals - Fixed Time Isolated





Site: 101 [LON\_RIVP(P) AM]

**♦** Network: N101 [Proposed AM (Proposed Arrangements)]

Longueville Rd, River Rd & Northwood Rd, Lane Cove 

Move	Movement Performance - Vehicles													
Mov	OD	Demand	Flows	Arrival	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average	
ID	Mov	Total	HV	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop	Speed	
												Rate		
		veh/h		veh/h	%	v/c	sec		veh	m		per veh	km/h	
South	: Northy	vood Rd (S	)											
1a	L1	621	8.0	621	8.0	0.384	2.9	LOS A	2.1	15.0	0.07	0.50	45.5	
2	T1	288	1.0	288	1.0	0.839	62.4	LOS E	6.9	49.0	1.00	0.92	2.3	
Appro	ach	909	0.9	909	0.9	0.839	21.7	LOS B	6.9	49.0	0.36	0.63	27.5	
North:	Longue	eville Rd (N	l)											
8	T1	261	0.8	261	8.0	0.477	36.7	LOS C	7.4	52.2	0.88	0.74	4.0	
9b	R3	43	0.0	43	0.0	0.511	65.3	LOS E	2.5	17.7	1.00	0.75	19.6	
Appro	ach	304	0.7	304	0.7	0.511	40.8	LOS C	7.4	52.2	0.90	0.75	8.5	
North\	West: R	iver Rd We	st (NW	<b>'</b> )										
27b	L3	179	6.1	179	6.1	0.861	28.5	LOS C	47.2	334.0	0.89	0.91	28.9	
29a	R1	1293	0.1	1293	0.1	0.861	31.1	LOS C	47.2	334.0	0.89	0.93	27.1	
Appro	ach	1472	8.0	1472	8.0	0.861	30.8	LOS C	47.2	334.0	0.89	0.92	27.3	
All Ve	hicles	2685	0.8	2685	8.0	0.861	28.8	LOS C	47.2	334.0	0.71	0.80	25.6	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 5.0 %

Number of Iterations: 10 (maximum specified: 10)

Move	Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow	Average Delay		Average Back Pedestrian	of Queue Distance	Prop.	Effective Stop Rate						
טו	Booshpaon	ped/h	sec	Service	ped	m	Queueu	per ped						
P3	North Full Crossing	6	11.4	LOS B	0.0	0.0	0.45	0.45						
P7	NorthWest Full Crossing	17	51.2	LOS E	0.1	0.1	0.95	0.95						
All Pe	destrians	23	40.8	LOS E			0.82	0.82						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Project: Z:\DATA\Data\Jobs\17work\17311\_266LonguevilleRdLaneCove\SIDRA\180824\Proposed Volumes (Proposed Layout).sip7



Site: 101 [LON\_RIVP(P) PM]

**♦** Network: N101 [Proposed PM (Proposed Arrangements)]

Longueville Rd, River Rd & Northwood Rd, Lane Cove 

Move	Movement Performance - Vehicles													
Mov	OD	Demand I		Arrival		Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average	
ID	Mov	Total	HV	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued		Speed	
		veh/h		veh/h	%	v/c	sec		veh	m		per veh	km/h	
South	: Northy	vood Rd (S	)											
1a	L1	1279	0.1	1279	0.1	0.691	12.8	LOSA	7.0	49.0	0.64	0.76	37.0	
2	T1	289	1.4	289	1.4	0.691	37.4	LOS C	6.9	49.0	1.00	0.88	3.6	
Appro	ach	1568	0.3	1568	0.3	0.691	17.4	LOS B	7.0	49.0	0.71	0.78	32.0	
North:	Longue	eville Rd (N	)											
8	T1	318	0.9	318	0.9	0.319	13.4	LOSA	7.4	52.2	0.62	0.54	9.5	
9b	R3	45	0.0	45	0.0	0.413	49.7	LOS D	2.0	14.1	1.00	0.74	23.2	
Appro	ach	363	0.8	363	8.0	0.413	17.9	LOS B	7.4	52.2	0.67	0.56	15.3	
North\	West: R	iver Rd We	st (NW	/)										
27b	L3	84	3.6	84	3.6	0.711	32.5	LOS C	17.5	123.6	0.92	0.85	27.2	
29a	R1	614	0.5	614	0.5	0.711	31.9	LOS C	17.5	123.6	0.92	0.87	26.7	
Appro	ach	698	0.9	698	0.9	0.711	32.0	LOS C	17.5	123.6	0.92	0.86	26.8	
All Ve	hicles	2629	0.5	2629	0.5	0.711	21.3	LOS B	17.5	123.6	0.76	0.77	29.0	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 145.2 %

Number of Iterations: 10 (maximum specified: 10)

Move	Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped						
P3	North Full Crossing	3	23.3	LOS C	0.0	0.0	0.73	0.73						
P7 All Pe	NorthWest Full Crossing destrians	10 13	38.2 34.8	LOS D	0.0	0.0	0.93	0.93						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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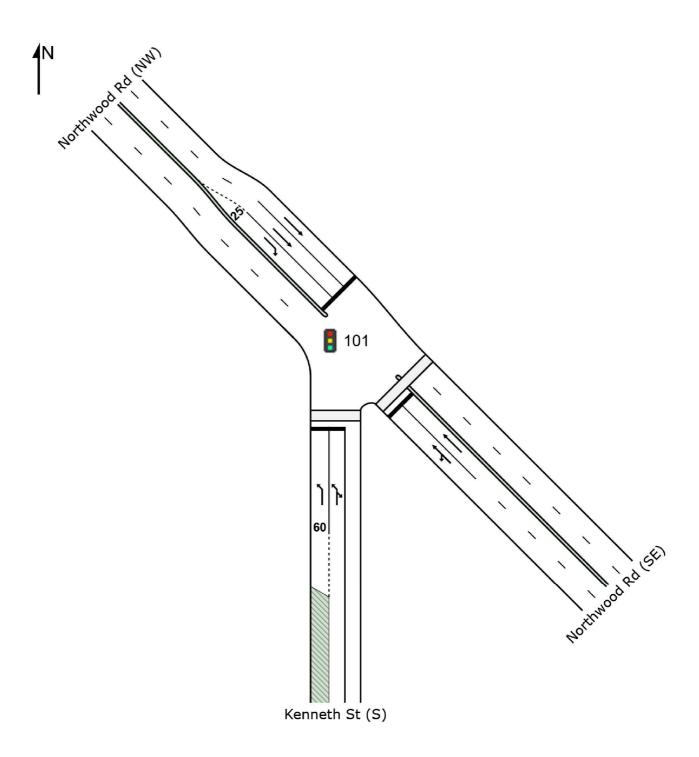
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# **SITE LAYOUT**

# Site: 101 [NOR\_KENX AM]

Northwood Rd & Kenneth St Signals - Fixed Time Isolated





Site: 101 [NOR\_KENX AM]

**♦** Network: N101 [Existing AM (Existing Arrangements)]

Northwood Rd & Kenneth St

Signals - Fixed Time Coordinated Cycle Time = 112 seconds (Network Cycle Time - Program)

Move	Movement Performance - Vehicles													
Mov	OD	Demand	Flows	Arrival	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective A	verage	
ID	Mov	Total	HV	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop S	Speed	
												Rate		
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h	
South	: Kenne	eth St (S)												
1a	L1	164	1.2	164	1.2	0.196	24.1	LOS B	5.4	38.5	0.65	0.70	30.2	
3b	R3	104	1.9	104	1.9	0.922	78.4	LOS F	6.9	49.0	1.00	1.04	23.9	
Appro	ach	268	1.5	268	1.5	0.922	45.2	LOS D	6.9	49.0	0.79	0.83	26.4	
South	East: N	orthwood F	Rd (SE)											
21b	L3	23	4.3	23	4.3	0.597	29.8	LOS C	21.3	150.0	0.80	0.72	37.3	
22	T1	761	8.0	761	8.0	0.597	24.6	LOS B	21.3	150.0	0.80	0.71	29.9	
Appro	ach	784	0.9	784	0.9	0.597	24.7	LOS B	21.3	150.0	0.80	0.71	30.2	
North'	West: N	Iorthwood F	Rd (NW	<b>'</b> )										
28	T1	1473	0.1	1473	0.1	0.521	3.4	LOSA	7.0	49.0	0.33	0.31	46.0	
29a	R1	161	0.6	161	0.6	0.786	42.6	LOS D	7.0	49.0	0.84	0.88	23.9	
Appro	ach	1634	0.2	1634	0.2	0.786	7.3	LOS A	7.0	49.0	0.38	0.36	42.1	
All Ve	hicles	2686	0.5	2686	0.5	0.922	16.1	LOS B	21.3	150.0	0.55	0.51	35.5	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 32.5 %

Number of Iterations: 10 (maximum specified: 10)

Move	ement Performance - Pedest	rians						
Mov ID	Description	Demand Flow	Average Delay		Average Back Pedestrian	of Queue Distance	Prop. Queued	Effective Stop Rate
		ped/h	sec		ped	m		per ped
P1	South Full Crossing	14	20.7	LOS C	0.0	0.0	0.61	0.61
P5	SouthEast Full Crossing	12	50.2	LOS E	0.0	0.0	0.95	0.95
All Pe	edestrians	26	34.3	LOS D			0.76	0.76

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 101 [NOR\_KENX PM]

++ Network: N101 [Existing PM (Existing Arrangements)]

Northwood Rd & Kenneth St

Signals - Fixed Time Coordinated Cycle Time = 87 seconds (Network Cycle Time - Program)

Move	ement l	Performa	nce - \	/ehicle	s								
Mov	OD	Demand	Flows	Arrival	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective A	verage
ID	Mov	Total	HV	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop	Speed
												Rate	
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h
South	: Kenne	eth St (S)											
1a	L1	168	2.4	168	2.4	0.275	27.3	LOS B	5.3	38.1	0.79	0.75	28.7
3b	R3	71	2.8	71	2.8	0.786	56.4	LOS D	3.4	24.6	1.00	0.91	27.9
Appro	ach	239	2.5	239	2.5	0.786	35.9	LOS C	5.3	38.1	0.85	0.79	28.4
South	East: N	orthwood F	Rd (SE)										
21b	L3	21	0.0	21	0.0	0.870	31.7	LOS C	39.4	276.1	0.94	0.96	36.6
22	T1	1346	0.1	1346	0.1	0.870	29.5	LOS C	39.4	276.1	0.94	1.00	27.7
Appro	ach	1367	0.1	1367	0.1	0.870	29.5	LOS C	39.4	276.1	0.94	1.00	27.9
North'	West: N	Iorthwood F	Rd (NW	<b>'</b> )									
28	T1	730	0.5	730	0.5	0.241	2.3	LOSA	3.9	27.3	0.26	0.23	47.2
29a	R1	194	0.5	194	0.5	0.895	51.1	LOS D	7.0	49.0	1.00	0.98	21.7
Appro	ach	924	0.5	924	0.5	0.895	12.6	LOSA	7.0	49.0	0.42	0.39	37.8
All Ve	hicles	2530	0.5	2530	0.5	0.895	23.9	LOS B	39.4	276.1	0.74	0.75	30.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 19.6 %

Number of Iterations: 10 (maximum specified: 10)

Move	ement Performance - Pedesti	rians						
Mov ID	Description	Demand Flow	Average Delay		Average Back Pedestrian	of Queue Distance	Prop. Queued	Effective Stop Rate
		ped/h	sec		ped	m		per ped
P1	South Full Crossing	2	12.7	LOS B	0.0	0.0	0.54	0.54
P5	SouthEast Full Crossing	28	37.8	LOS D	0.1	0.1	0.93	0.93
All Pe	destrians	30	36.1	LOS D			0.91	0.91

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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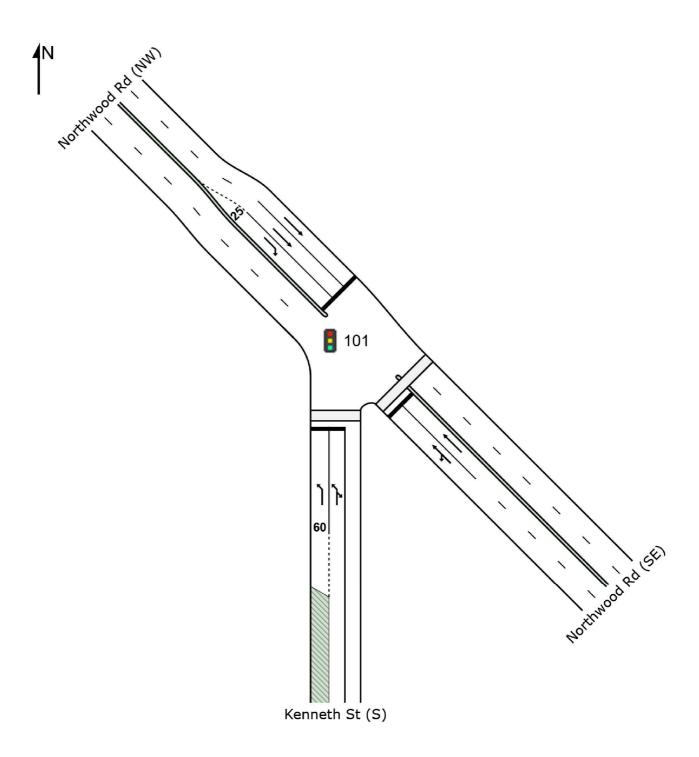
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# **SITE LAYOUT**

# Site: 101 [NOR\_KENP(P) AM]

Northwood Rd & Kenneth St Signals - Fixed Time Isolated





Site: 101 [NOR\_KENP(P) AM]

**♦** Network: N101 [Proposed AM (Proposed Arrangements)]

Northwood Rd & Kenneth St

Signals - Fixed Time Coordinated Cycle Time = 114 seconds (Network Cycle Time - Program)

Move	ement	Performa	nce - \	/ehicle	s								
Mov	OD	Demand	Flows	Arrival	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
ID	Mov	Total	HV	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h
South	: Kenne	eth St (S)											
1a	L1	164	1.2	164	1.2	0.174	20.6	LOS B	5.0	35.3	0.59	0.68	32.1
3b	R3	104	1.9	104	1.9	0.938	82.6	LOS F	7.2	50.9	1.00	1.06	23.3
Appro	ach	268	1.5	268	1.5	0.938	44.7	LOS D	7.2	50.9	0.75	0.83	26.5
South	East: N	lorthwood F	Rd (SE)	)									
21b	L3	23	4.3	23	4.3	0.682	35.4	LOS C	24.2	170.7	0.88	0.79	35.2
22	T1	771	0.8	771	8.0	0.682	30.5	LOS C	24.2	170.7	0.88	0.79	27.3
Appro	ach	794	0.9	794	0.9	0.682	30.6	LOS C	24.2	170.7	0.88	0.79	27.6
North\	West: N	lorthwood F	Rd (NW	/)									
28	T1	1486	0.1	1486	0.1	0.514	3.1	LOSA	7.0	49.0	0.31	0.29	46.3
29a	R1	145	0.7	145	0.7	0.682	39.2	LOS C	7.0	49.0	0.92	0.80	25.0
Appro	ach	1631	0.2	1631	0.2	0.682	6.3	LOSA	7.0	49.0	0.37	0.33	43.0
All Ve	hicles	2693	0.5	2693	0.5	0.938	17.3	LOS B	24.2	170.7	0.56	0.51	34.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 5.0 %

Number of Iterations: 10 (maximum specified: 10)

Move	ement Performance - Pedesti	rians						
Mov ID	Description	Demand Flow	Average Delay		Average Back Pedestrian	of Queue Distance	Prop. Queued	Effective Stop Rate
		ped/h	sec		ped	m		per ped
P1	South Full Crossing	14	24.7	LOS C	0.0	0.0	0.66	0.66
P5	SouthEast Full Crossing	12	51.2	LOS E	0.0	0.0	0.95	0.95
All Pe	destrians	26	36.9	LOS D			0.79	0.79

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 101 [NOR\_KENP(P) PM]

**♦** Network: N101 [Proposed PM (Proposed Arrangements)]

Northwood Rd & Kenneth St

Signals - Fixed Time Coordinated Cycle Time = 88 seconds (Network Cycle Time - Program)

Move	ement l	Performa	nce - \	/ehicle	s								
Mov	OD	Demand	Flows	Arrival	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
ID	Mov	Total	HV	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop	Speed
												Rate	
0 11	1.6	veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h
South	: Kenne	eth St (S)											
1a	L1	168	2.4	168	2.4	0.288	28.7	LOS C	5.5	39.4	0.80	0.75	28.1
3b	R3	71	2.8	71	2.8	0.795	57.3	LOS E	3.5	25.0	1.00	0.91	27.8
Appro	ach	239	2.5	239	2.5	0.795	37.2	LOS C	5.5	39.4	0.86	0.80	28.0
0 11			)										
South	East: N	orthwood F	Ra (SE)										
21b	L3	21	0.0	21	0.0	0.852	28.3	LOS B	37.5	262.7	0.91	0.91	37.9
22	T1	1358	0.1	1358	0.1	0.852	25.8	LOS B	37.5	262.7	0.91	0.94	29.3
Appro	ach	1379	0.1	1379	0.1	0.852	25.9	LOS B	37.5	262.7	0.91	0.94	29.5
North\	West: N	Iorthwood F	Rd (NW	<b>/</b> )									
28	T1	741	0.5	741	0.5	0.243	1.0	LOSA	1.9	13.5	0.10	0.09	48.7
29a	R1	175	0.6	175	0.6	0.866	50.3	LOS D	7.0	49.0	1.00	0.94	21.9
Appro	ach	916	0.5	916	0.5	0.866	10.4	LOSA	7.0	49.0	0.27	0.25	39.4
All Ve	hicles	2534	0.5	2534	0.5	0.866	21.4	LOS B	37.5	262.7	0.68	0.68	32.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 145.2 %

Number of Iterations: 10 (maximum specified: 10)

Move	ement Performance - Pedesti	rians						
Mov ID	Description	Demand Flow	Average Delay		Average Back Pedestrian	of Queue Distance	Prop. Queued	Effective Stop Rate
		ped/h	sec		ped	m		per ped
P1	South Full Crossing	2	12.0	LOS B	0.0	0.0	0.52	0.52
P5	SouthEast Full Crossing	28	38.2	LOS D	0.1	0.1	0.93	0.93
All Pe	destrians	30	36.5	LOS D			0.91	0.91

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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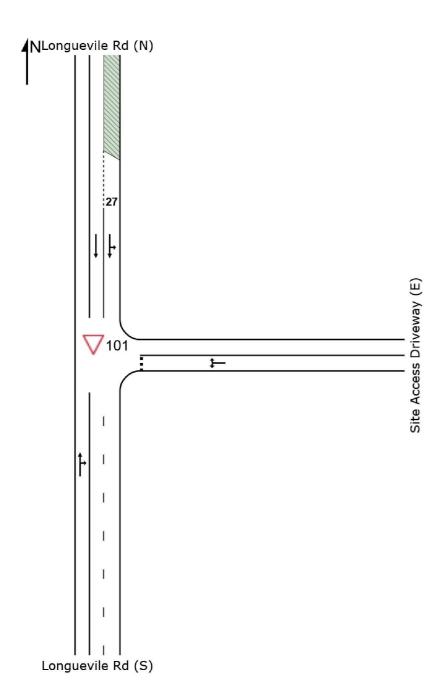
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# **SITE LAYOUT**

# Site: 101 [LON\_ACCX AM]

Longueville Rd & Site Access Driveway Giveway / Yield (Two-Way)





Longueville Rd & Site Access Driveway Giveway / Yield (Two-Way)

♦♦ Network: N101 [Existing AM (Existing Arrangements)]

Move	ement l	Performai	nce - \	/ehicle	s								
Mov	OD	Demand	Flows	Arrival	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
ID	Mov	Total	HV	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued		Speed
												Rate	
0 "		veh/h		veh/h	%	v/c	sec		veh	m		per veh	km/h
South	i: Longu	evile Rd (S	5)										
2	T1	460	3.0	460	3.0	0.245	0.0	LOS A	0.0	0.1	0.00	0.00	50.0
3	R2	1_	0.0	1_	0.0	0.245	4.5	LOS A	0.0	0.1	0.00	0.00	21.5
Appro	ach	461	3.0	461	3.0	0.245	0.0	NA	0.0	0.1	0.00	0.00	49.8
East:	Sito Acc	ess Drivev	νον (Ε)										
			, ,										
4	L2	3	0.0	3	0.0	0.012	0.4	LOS A	0.0	0.2	0.33	0.22	19.4
6	R2	3	0.0	3	0.0	0.012	5.2	LOS A	0.0	0.2	0.33	0.22	27.6
Appro	ach	6	0.0	6	0.0	0.012	2.8	LOS A	0.0	0.2	0.33	0.22	24.2
North	: Longue	evile Rd (N	)										
7	L2	1	0.0	1	0.0	0.071	4.6	LOSA	0.4	2.8	0.00	0.00	49.5
8	T1	270	1.9	270	1.9	0.071	0.0	LOSA	0.4	2.8	0.00	0.00	50.0
Appro	ach	271	1.8	271	1.8	0.071	0.0	NA	0.4	2.8	0.00	0.00	50.0
All Ve	hicles	738	2.6	738	2.6	0.245	0.0	NA	0.4	2.8	0.00	0.00	49.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 32.5  $\,\%$ 

Number of Iterations: 10 (maximum specified: 10)

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♦♦ Network: N101 [Existing PM (Existing Arrangements)]

Longueville Rd & Site Access Driveway Giveway / Yield (Two-Way)

Move	ement l	Performan	ice - \	/ehicle	s								
Mov ID	OD Mov	Demand I Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h		veh/h	%	v/c	sec		veh	m		per veh	km/h
South	: Longu	evile Rd (S)	)										
2	T1	369	1.9	369	1.9	0.197	0.0	LOSA	0.0	0.1	0.00	0.00	49.9
3	R2	2	0.0	2	0.0	0.197	4.8	LOS A	0.0	0.1	0.00	0.00	21.5
Appro	ach	371	1.9	371	1.9	0.197	0.0	NA	0.0	0.1	0.00	0.00	49.6
East:	Site Acc	cess Drivew	ay (E)										
4	L2	1	0.0	1	0.0	0.005	0.4	LOS A	0.0	0.1	0.40	0.26	19.3
6	R2	2	0.0	2	0.0	0.005	4.7	LOS A	0.0	0.1	0.40	0.26	27.5
Appro	ach	3	0.0	3	0.0	0.005	3.3	LOSA	0.0	0.1	0.40	0.26	25.3
North:	: Longue	evile Rd (N)											
7	L2	3	0.0	3	0.0	0.094	4.6	LOSA	0.0	0.0	0.00	0.01	49.4
8	T1	326	1.8	326	1.8	0.094	0.0	LOSA	0.0	0.0	0.00	0.00	49.9
Appro	ach	329	1.8	329	1.8	0.094	0.1	NA	0.0	0.0	0.00	0.01	49.9
All Ve	hicles	703	1.8	703	1.8	0.197	0.1	NA	0.0	0.1	0.00	0.01	49.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 19.6  $\,\%$ 

Number of Iterations: 10 (maximum specified: 10)

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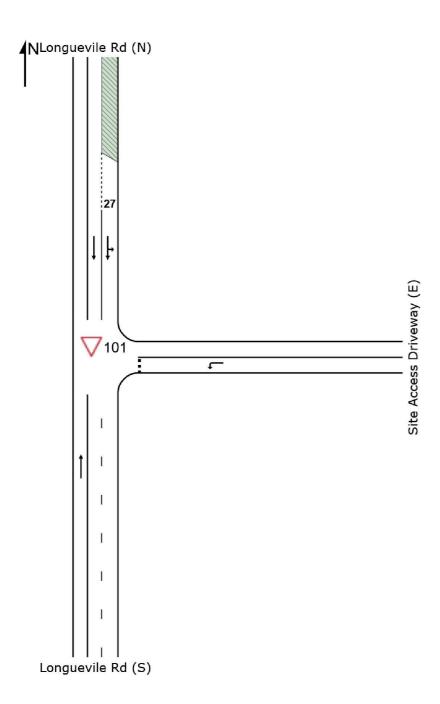
Organisation: VARGA TRAFFIC PLANNING | Processed: Friday, 24 August 2018 10:26:11 AM

Project: Z:\DATA\Data\Jobs\17work\17311\_266LonguevilleRdLaneCove\SIDRA\180824\Existing Volumes (Existing Layout).sip7

# **SITE LAYOUT**

# Site: 101 [LON\_ACCP(P) AM]

Longueville Rd & Site Access Driveway Giveway / Yield (Two-Way)





**♦** Network: N101 [Proposed AM (Proposed Arrangements)] Longueville Rd & Site Access Driveway

Giveway / Yield (Two-Way)

Move	ment l	Performa	nce - \	/ehicle	s								
Mov ID	OD Mov	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective A	Average Speed
	10100						Dolay	0011100	VOTITOTOS	Distance	Queucu	Rate	Ороса
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h
South	: Longu	evile Rd (S	S)										
2	T1	460	3.0	460	3.0	0.244	0.0	LOSA	0.0	0.0	0.00	0.00	50.0
Appro	ach	460	3.0	460	3.0	0.244	0.0	NA	0.0	0.0	0.00	0.00	50.0
East:	Site Aco	cess Drivev	vay (E)										
4	L2	37	0.0	37	0.0	0.025	0.3	LOS A	0.1	0.7	0.21	0.08	19.8
Appro	ach	37	0.0	37	0.0	0.025	0.3	LOSA	0.1	0.7	0.21	0.08	19.8
North:	Longu	evile Rd (N	l)										
7	L2	30	0.0	30	0.0	0.079	4.6	LOSA	3.6	25.3	0.00	0.11	48.9
8	T1	270	1.9	270	1.9	0.079	0.0	LOSA	3.6	25.3	0.00	0.05	49.4
Appro	ach	300	1.7	300	1.7	0.079	0.5	NA	3.6	25.3	0.00	0.05	49.3
All Ve	hicles	797	2.4	797	2.4	0.244	0.2	NA	3.6	25.3	0.01	0.02	46.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 5.0 %

Number of Iterations: 10 (maximum specified: 10)

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Longueville Rd & Site Access Driveway Giveway / Yield (Two-Way)

**♦** Network: N101 [Proposed PM (Proposed Arrangements)]

Move	ment l	Performa	nce - \	/ehicle	s								
Mov ID	OD Mov	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		per veh	km/h
South	: Longu	evile Rd (S	5)										
2	T1	369	1.9	369	1.9	0.195	0.0	LOSA	0.0	0.0	0.00	0.00	50.0
Appro	ach	369	1.9	369	1.9	0.195	0.0	NA	0.0	0.0	0.00	0.00	50.0
East:	Site Aco	cess Drivev	vay (E)										
4	L2	33	0.0	33	0.0	0.023	0.4	LOSA	0.1	0.6	0.23	0.10	19.8
Appro	ach	33	0.0	33	0.0	0.023	0.4	LOS A	0.1	0.6	0.23	0.10	19.8
North:	Longu	evile Rd (N	)										
7	L2	36	0.0	36	0.0	0.095	4.6	LOSA	0.4	2.9	0.00	0.11	48.9
8	T1	326	1.8	326	1.8	0.095	0.0	LOSA	0.4	2.9	0.00	0.05	49.4
Appro	ach	362	1.7	362	1.7	0.095	0.5	NA	0.4	2.9	0.00	0.05	49.3
All Ve	hicles	764	1.7	764	1.7	0.195	0.2	NA	0.4	2.9	0.01	0.03	46.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 145.2 %

Number of Iterations: 10 (maximum specified: 10)

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